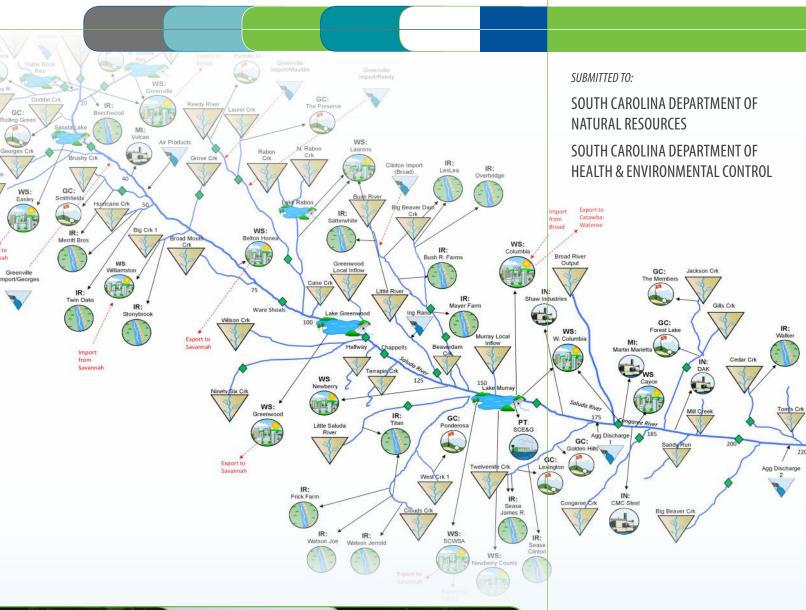
SOUTH CAROLINA SURFACE WATER QUANTITY MODELS SALUDA RIVER BASIN MODEL





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PREPARED BY:





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Purpose

This document, the Saluda River Basin Modeling Report, is provided in support of the Surface Water Availability Assessment for the South Carolina Department of Natural Resources (DNR) and the South Carolina Department of Health and Environmental Control (DHEC). The Surface Water Availability Assessment is part of a broader strategy to augment statewide water planning tools and policies, culminating in the development of regional water plans and the update of the State Water Plan.

The Surface Water Availability Assessment focuses on the development of surface water quantity models. The models are primarily intended to represent the impacts of water withdrawals, return flows, and storage on the usable and reliably available water quantity throughout each major river basin in the state. With this ability, they will be used for regional water planning and management, policy evaluation and permit assessments.

This Saluda River Basin Modeling Report presents the model objectives; identifies revisions made to the initial model framework; summarizes model inputs and assumptions; presents the calibration approach and results; and provides guidelines for model use. Further guidance on use of the Saluda River Basin Model is provided in the *Simplified Water Allocation Model (SWAM) User's Manual* (CDM Smith, 2015).

Additionally, this document is intended to help disseminate the information about how the model represents the Saluda River Basin to parties with a vested interest in water management (stakeholders). To this end, the language is intended to be accessible and explanatory, describing the model development process in clear English without undue reliance on mathematical formulations, programming nuances, or modeling vernacular.



Modeling Objectives

The Saluda River Basin Model in SWAM has been developed for multiple purposes, but it is primarily intended to support future permitting, policy, and planning efforts throughout the basin. Fundamentally, the model will simulate the natural hydrology through the network of the Saluda River and its major tributaries, and the impacts to the river flows from human intervention: withdrawals, discharges, impoundment, and interbasin transfers.

The model will simulate historic hydrologic conditions from 1925 through 2013. Defining and developing this hydrologic period of record required numerous assumptions and estimations of past flow and water use patterns, which were vetted during the calibration process. The purpose of the models is not to reproduce with high accuracy the flow on any given day in history. Rather, the purpose is to reproduce with confidence the frequency at which natural and managed flows have reached any given threshold, and by extension, how they might reach these thresholds under future use conditions. To this end, one important objective of model formulation was to reproduce hydrologic peaks and low flows on a monthly and daily basis, recession patterns on a monthly and daily basis, and average flows over months and years.

The end goals of the model are derived specifically from the project scope. The intended uses include:

- 1. Evaluate surface-water availability in support of the Surface Water Withdrawal, Permitting, Use, and Reporting Act;
- 2. Predict future surface-water availability using projected demands;
- 3. Develop regional water-supply plans;
- 4. Test the effectiveness of new water-management strategies or new operating rules; and
- 5. Evaluate the impacts of future withdrawals on instream flow needs and minimum instream flows as defined by regulation.

Lastly, the model is intended to support a large user base, including staff at DNR and DHEC along with stakeholders throughout the Saluda River Basin. To this end, the master file will be maintained on a cloud-based server, and will be made accessible to trained users through agreement with DNR and/or DHEC. To support its accessibility, the SWAM model interface is designed to be visual and intuitive, but using the model and extracting results properly will require training for any future user.



Review of the Modeling Plan

The modeling approach, data requirements, software, and resolution are described in the *South Carolina Surface Water Quantity Models - Modeling Plan*, (CDM Smith, November 2014).

The Modeling Plan is an overarching approach, intended to guide the development of all eight river basin models for South Carolina by describing consistent procedures, guidelines, and assumptions that will apply to each basin and model. It is not an exhaustive step-by-step procedure for developing a model in SWAM, nor does this address all of the specific issues that may be unique to particular basins. Rather, the Modeling Plan offers strategic guidelines aimed at helping model development staff make consistent judgments and decisions regarding model resolution, data input, and representation of operational variables and priorities.

The Modeling Plan was followed during development of the Saluda River Basin Model. Where appropriate, additional discussion has been included in this report, to elaborate on specific aspects covered in the Modeling Plan.



Saluda Model Framework

The initial Saluda River Basin SWAM Model Framework was developed in collaboration with South Carolina DNR and DHEC, and was presented in the memorandum *Saluda Basin SWAM Model Framework* (CDM Smith, March 2015). The proposed framework was developed as a starting point for representing the Saluda Basin river network and its significant water withdrawals and discharges. The guiding principles in determining what elements of the Saluda River Basin to simulate explicitly were:

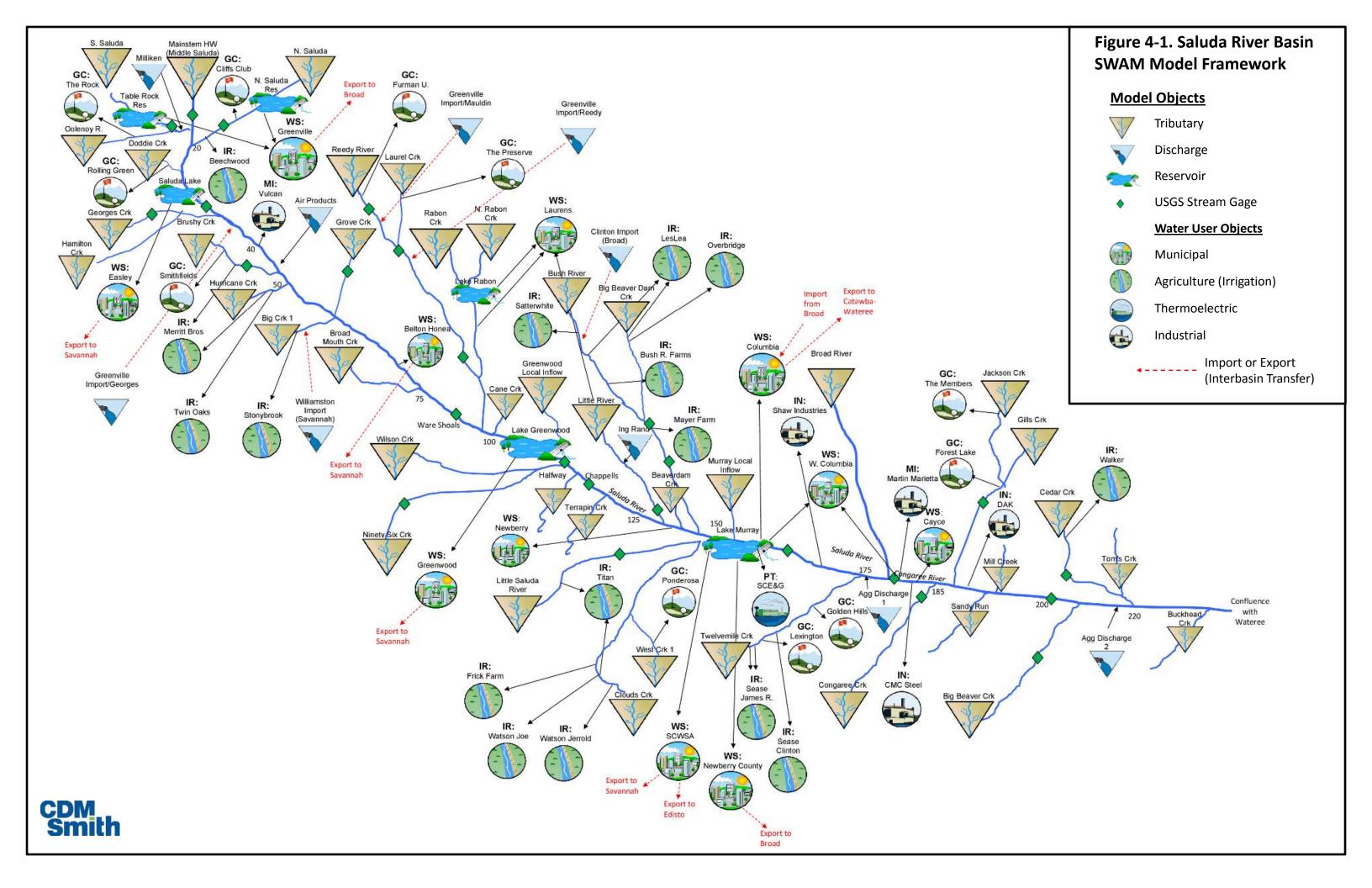
- 1. Begin with a simple representation, with the understanding that it is easier to add additional details in the future than to remove unnecessary detail to make the model more efficient.
- 2. Incorporate all significant withdrawals and discharges. Significant withdrawals include those that have a permit or registration which indicated that they may withdrawal over 3 million gallons in any month. Significant discharges are those that average over 3 million gallons per month (mg/month). In some instances, discharges that average less than 3 mg/month were included, such as discharges directly associated with a permitted or registered withdrawal.
- 3. Any tributary with current uses (permitted or registered withdrawals or significant discharge) will be represented explicitly. This includes most primary tributaries to the Saluda and its major branches, and some secondary tributaries.
- 4. Generally, tributaries that are unused are not included explicitly, but the hydrologic contributions from these tributaries is embedded in the unimpaired flows (or reach gains) in downstream locations. As unimpaired flows (UIFs) are developed throughout the Saluda, some additional tributaries may be added explicitly if warranted as candidates to support future use (or these can be easily added at any time in the future as permit applications are received).

During model development, simplifications were made in some areas, while more detail was added in others. **Figure 4-1** visually depicts the SWAM model framework, including tributaries, water users, and dischargers. As the framework is presented in the following paragraphs, changes made to the original model framework are noted.

4.1 Representation of Water Withdrawals

As noted above, significant withdrawals include those that have a permit or registration – which indicated that they may withdraw over 3 million gallons in any month. For several of the municipal water users represented in Saluda Model, withdrawal data includes both water used directly by that water user and water sold to other major municipal water users who are included as separate objects in the model. For example, permit #23WS002 associated with the Greenville Water User object, includes water used directly by Greenville as well as water sold to Easley Combined Utilities, who has their own withdrawal permit. Greenville water also sells water to other smaller systems.





Based on feedback from DNR, DHEC, and the Technical Advisory Committee (TAC), the decision was made to represent water withdrawals based on the permit holder rather than the ultimate water user. In this regard, the Water User objects reflect the withdrawals associated with their permit. In the example above, the water purchased by Easley Combined Utilities from Greenville is accounted for under Greenville's Water User object. The alternative approach would have been to associate all of Easley Combined Utilities' demand as part of their own Water User object, including the water purchased from Greenville. The disadvantage of this approach is that the withdrawal permits associated with these conditions would be somewhat disaggregated in the model. Changes to a single permit limit, for example, would need to be applied for multiple users in the model. For this reason, the permit-based approach was selected for representing water withdrawals.

4.2 Representation of Discharges

Water and wastewater discharges can be simulated two ways in SWAM. First, they can be associated with a Water User object, each of which may specify five points of discharge anywhere in the river network. These discharges are not represented with visual model objects, but are identified within the dialogue box for the associated Water User object. Alternatively, discharges can be specified within a Discharge object. There are advantages and disadvantages with both methods. Associating discharges with withdrawals helps to automatically maintain a reasonable water balance because discharges are specified as seasonally-variable percentage of the withdrawal. However, it may be more difficult to test a maximum discharge permit level using this approach. Alternatively, using a tributary object to specify outflows allows for more precise representation of discharge variability, but does not automatically preserve the water balance (the user will need to adjust withdrawals to match simulated discharge). This second approach is also appropriate for interbasin transfers, in which source water resides in another basin but is discharged in the basin represented by the model.

In the Saluda River Basin Model, discharges are most often represented within the Water User object. The several exceptions, where a Discharge object was used, include the following:

- Several industrial discharges were deemed significant enough to include in the model; however, these industries either purchase water from another permit holder or withdraw (or supplement) using groundwater. They did not have their own surface water withdrawal permit. These include: Milliken, Air Products and Ingersoll Rand.
- Below Lake Murray, several small municipal and industrial discharges were aggregated together based on their close proximity, and are represented by two Discharge objects. These inlcude Bush River, CWS/Watergate, Woodland Hills, CWS/I-20, and CWS/Friarsgate, which are represented by the Agg Discharge 1 object; and Devro and Westinghouse which are represented by the Agg Discharge 2 object. None of these dischargers have their own surface water withdrawal permit.
- Water withdrawn by Greenville Water from Lake Keowee in the Savannah Basin, and then
 discharged in the Saluda Basin is represented by three separate Discharge objects. These
 discharge objects represent wastewater discharges by Renewable Water Resources (ReWa) at
 their Mauldin Road, Georges Creek, and Lower Reedy River wastewater treatment facilities.
- Water withdrawn by the City of Clinton in the Broad River Basin, and then discharged in the Saluda Basin is represented by a Discharge object.



4.3 Representation of Hydropower Facilities

In the original model framework, the hydropower facilities in the Saluda Basin were represented with Instream Flow objects. The use of an Instream Flow object allows for the inclusion of a minimum release which can be prioritized or at least closely tracked in the model. As operational information was collected for each hydropower facility, it became clear that most of the facilities in the Saluda operate essentially as run-of-river facilities where inflow equals outflow on an instantaneous basis. Since these run-of-river hydropower facilities neither impact the water balance (no storage) nor have associated flow requirements or consumption, they can be ignored in the model framework. Therefore, the following hydropower facilities were removed from the framework:

- Upper Pelzer Hydro
- Lower Pelzer Hydro
- Holiday Bridge Hydro
- Ware Shoals Hydro
- Boyd's Mill Hydro
- Saluda Hydro (on Saluda Lake)

The Saluda Dam and Hydro on Lake Murray and Buzzard's Roost Hydro on Lake Greenwood are the two facilities that are not run-of-river. Each facility has minimum flow requirements and unique release/operating rules, which are discussed further in Section 6. The rules for these two facilities are specified within the Lake Murray and Lake Greenwood reservoir objects.

4.4 Groundwater Users and Associated Discharge

Although the Saluda Model focuses on surface water, representation of groundwater withdrawal (demand) within the model can be useful when the return flows, which are greater than 3 mg/month, are to surface water. In these cases, representation of the groundwater withdrawal by a Water User object, especially for municipalities, is useful because the (monthly) discharge percentage is specified with the Water User object. Since model scenarios typically focus on changes to water demand/use, the user can simply update the demand (in the Water User object, "Water Usage" tab), and the return flows will automatically be re-calculated. For water users who withdrawal groundwater, the "Groundwater" option is selected in the Source Water Type section of the "Source Water" tab.

In the Saluda Basin, no significant, municipal groundwater withdrawals were identified which had a corresponding, significant discharge to surface water; therefore, there are no groundwater users that are represented by a Water User object.

4.5 Implicit Tributaries

At certain locations along the main stem of the Saluda River, new implicit tributary objects were added to capture ungaged drainage areas and tributary inputs not included in the original model framework. The list of implicit tributaries included in the Saluda Model is provided in Section 6. These are tributaries which are not as likely to support future use as the explicitly represented tributaries; however, their contribution of flow to the main stem is important to include.



Model Versions

For each river basin, two model versions were developed: a calibration model and a baseline model. The two models have different objectives and purposes, and, consequently, employ different parameter assignments, as described below.

The calibration model was developed to determine the "best fit" value of key model hydrologic parameters, as described in Section 7. Its utility beyond the calibration exercise is limited as the calibration model has been developed to recreate historical conditions which are not necessarily representative of current or planned future conditions. This model was parameterized using historical water use and reservoir operations data to best reflect past conditions in the basin. These data include time-varying river and reservoir withdrawals and consumptive use estimates and historical reservoir release and operational rules. Also included in the calibration version of the model are water users that may be no longer active but were active during the selected calibration period. As discussed in Section 7, the simulation period for this version of the model focuses on the recent past (1983 – 2013) rather than the full record of estimated hydrology.

In contrast, the baseline model is intended to represent current demands and operations in the basin combined with an extended period of estimated hydrology. This model will serve as the starting point for any future predictive simulations with the model (e.g., planning or permitting support) and should be maintained as a useful "baseline" point of reference. For this model, the simulation period extends back to 1925, the start of the hydrologic record for the Saluda River Basin. Each element in the baseline model is assigned water use rates that reflect current demands only and are not time variable (except seasonal). Current demands were estimated by averaging water use data over the past ten years (2005 - 2014), on a monthly basis. These monthly demands are repeated in the baseline model for each simulation year. Similarly, reservoir operations defined in the baseline model are based on current rules, guidelines, and minimum release requirements. In certain instances, future rules that are not yet in effect, were include (and can be toggled on or off in the model). An example of a future rule is the required minimum release associated with the Lake Murray Striped Bass Flow Enhancement Flow Regime. This requirement is part of the Saluda Hydro Federal Energy Relicense (FERC), which is still pending. A final difference between the two models is that only active water users are included in the baseline model. Inactive user objects included in the calibration model have been removed from the baseline model.



Model Inputs

SWAM inputs include unimpaired flows (UIFs); reservoir characteristics such as operating rule curves, storage-area-relationships, and evaporation rates; and water user information, including withdrawals, consumptive use, and return flows. This section summarizes the inputs used in both the calibration and baseline Saluda River Basin Models. As explained in Section 5, the calibration model incorporates historical water withdrawal and return data so that UIF flows and reach gains and losses can be calibrated to USGS gage flows. In contrast, the baseline model represents current demands and operations in the basin combined with an extended period of estimated hydrology. For future uses of the model, users can adjust the inputs, including demands, permit limits, and operational strategies, to perform "what if" simulations of basin water availability.

The following subsections describe the specific inputs to the Saluda Model. Unless specifically noted, the inputs discussed below are the same in both the calibration model and baseline model.

6.1 Model Tributaries

The primary hydrologic inputs to the model are unimpaired flows for each tributary object. These flows, entered as a continuous timeseries of monthly and daily average data, represent either the flow at the top of each tributary object reach (headwater flows; explicit tributary objects) or at the bottom of the reach (confluence flows; implicit tributary objects). Additionally, mid-stream UIFs, though not used directly in the SWAM model construction, can serve as useful references in the model calibration process, particularly with respect to quantified reach gains and losses (discussed in Section 7).

6.1.1 Explicit Tributary Objects: Headwater Flows

Explicit tributary objects in SWAM are tributaries that include any number of Water User objects and/or reservoir objects with operations and water use explicitly simulated in the model. Conversely, implicit tributary objects (discussed below) are treated as simple point inflows to receiving streams in the model, without any simulated water use or operations. For further discussion on explicit versus implicit tributary objects in SWAM, please refer to the SWAM User's Manual.

Explicit tributary objects are parameterized in SWAM with headwater flows, representing unimpaired flows at the top of the given modeled reach. These flows may be raw gage flow, or area-prorated from calculated UIFs elsewhere in the basin. **Table 6-1** summarizes the gages, or in many instances, the reference gages used to develop headwater flows. **Figure 6-1** highlights the upstream drainage areas associated with the explicit tributary headwater flows. Green polygons correspond to unimpaired USGS gaged flow and purple polygons correspond to estimated ungaged flows. The inset table designates the project ID for each flow point, whether it was gaged or ungaged, the name of the tributary, and the corresponding drainage area in acres.

6.1.2 Implicit Tributary Objects: Confluence Flows

For implicit tributaries, all input confluence flows were estimated from reference UIFs. **Table 6-2** lists which unimpaired USGS gage was used as a reference gage for calculating flows for each implicit tributary object. **Figure 6-2** shows drainage areas for nine implicit tributaries and two local inflows



Table 6-1. Gages and Reference Gages Used for Headwater Flows on Explicit Tributaries

			Headwater Input	USGS R	Reference G	age (Unimpaired)
Project ID	Туре	USGS Number	SWAM Tributary	Project Gage ID	USGS Number	Stream
SLD129	Ungaged	-	Little Saluda River	SLD25	2168504	Saluda River
SLD139	Ungaged	-	Congaree Creek	SLD28	2169500	Congaree Creek
SLD200	Ungaged	-	North Saluda River	SLD03	21623975	North Saluda River
SLD201	Ungaged	-	South Saluda River	SLD01	2162290	South Saluda River
SLD203	Ungaged	-	Oolenoy River	SLD04	2162500	Saluda River
SLD204	Ungaged	-	Doddies Creek	SLD04	2162500	Saluda River
SLD205	Ungaged	-	Reedy River	SLD10	2164000	Reedy River
SLD206	Ungaged	-	Georges Creek	SLD06	2163000	Saluda River
SLD207	Ungaged	-	Brushy Creek	SLD33	2162700	Middle Branch
SLD210	Ungaged	-	Hurricane Creek	SLD06	2163000	Saluda River
SLD211	Ungaged	-	Big Creek	SLD09	2163500	Saluda River
SLD212	Ungaged	-	Laurel Creek	SLD11	2164110	Reedy River
SLD213	Ungaged	-	Broad Mouth Creek	SLD09	2163500	Saluda River
SLD215	Ungaged	-	Wilson Creek	SLD18	2167000	Saluda River
SLD216	Ungaged	-	Little River	SLD19	2167450	Little River
SLD218	Ungaged	-	Big Beaverdam Creek	SLD21	2167563	Bush River
SLD221	Ungaged	-	Clouds Creek	SLD25	2168504	Saluda River
SLD222	Ungaged	-	West Creek	SLD25	2168504	Saluda River
SLD223	Ungaged	-	Twelvemile Creek	SLD26	2169000	Saluda River
SLD225	Ungaged	-	Cedar Creek	SLD32	2169670	Cedar Creek
SLD226	Ungaged	-	Jackson Creek	SLD29	2169570	Gills Creek
SLD227	Ungaged	-	Gills Creek	SLD29	2169570	Gills Creek
SLD02	Gaged	2162350	Mainstem Saluda River (Middle Saluda)*	-	-	-
SLD05	Gaged	2162525	Hamilton Creek	-	-	-
SLD08	Gaged	21630967	Grove Creek	-	-	-
SLD14	Gaged	2165200	Rabon Creek (South Rabon)*	-	-	-
SLD15	Gaged	21652801	North Rabon Creek	-	-	-
SLD17	Gaged	2166970	Ninety-Six Creek	-	-	-
SLD31	Gaged	2169630	Big Beaver Creek	-	-	-
SLD34	Gaged	2167557	Bush River	-	-	-

^{*}Actual river name in parenthesis

Table 6-2. Reference Gages Used for Headwater Flows on Implicit Tributaries

	Ungaged Basin	USGS	Reference	Gage (Unimpaired)
Project ID	SWAM Tributary	Project Gage ID	USGS Number	Stream
SLD115	Turkey Creek	SLD16	2166501	Saluda River
SLD301	Lake Greenwood Inflow	SLD16	2166501	Saluda River
SLD161	Halfway Swamp Creek	SLD17	2166970	Ninety-Six Creek
SLD163	Terrapin Creek	SLD17	2166970	Ninety-Six Creek
SLD160	Cane Creek	SLD19	2167450	Little River
SLD162	Beaverdam Creek	SLD21	2167563	Bush River
SLD302	Lake Murray Inflow	SLD25	2168504	Saluda River
SLD164	Sandy Run	SLD32	2169670	Cedar Creek
SLD165	Mill Creek	SLD32	2169670	Cedar Creek
SLD166	Toms Creek	SLD32	2169670	Cedar Creek
SLD167	Buckhead Creek	SLD31	2169630	Big Beaver Creek



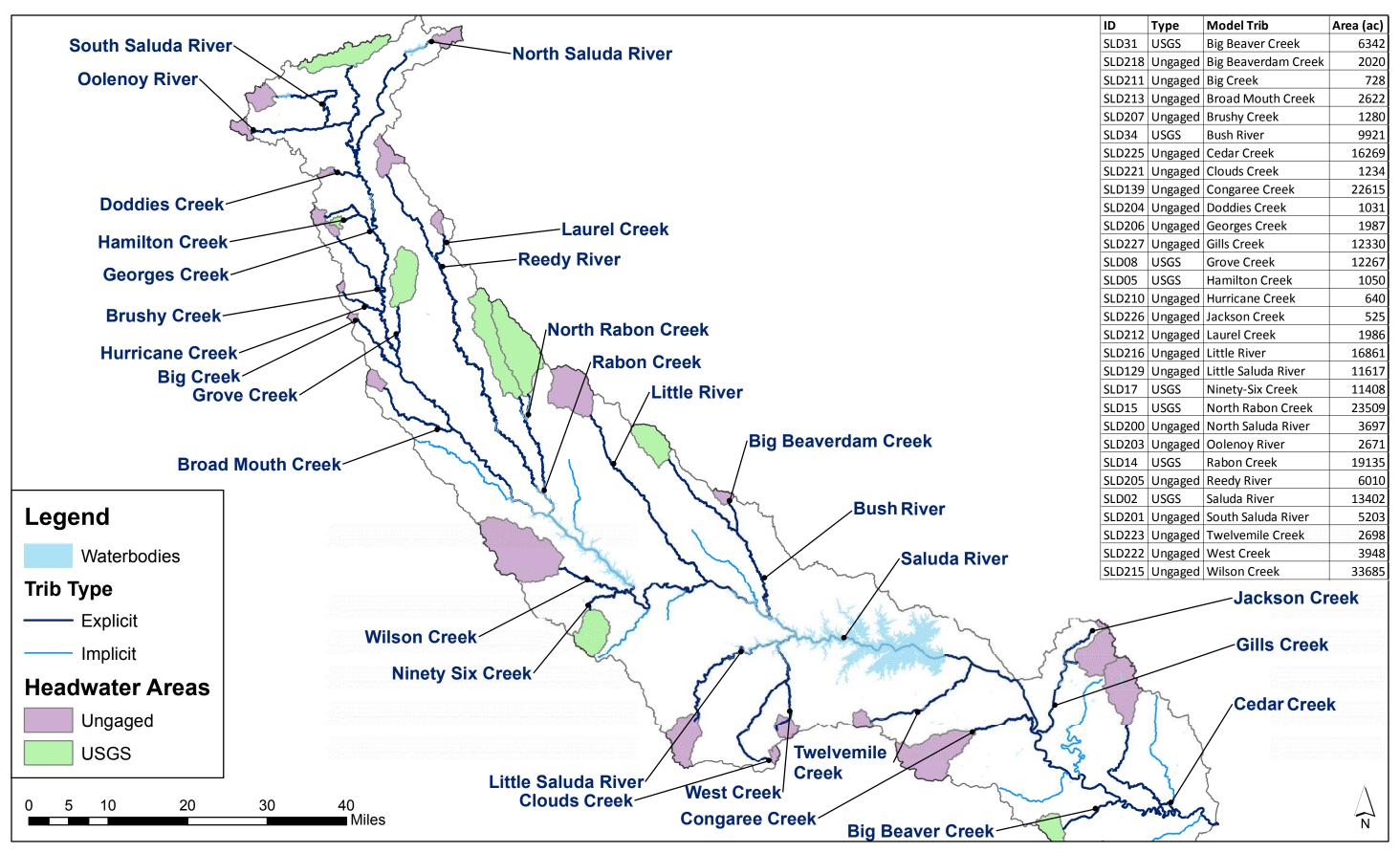




Figure 6-1. Headwater Areas for Explicit Tributaries in the Saluda River Basin

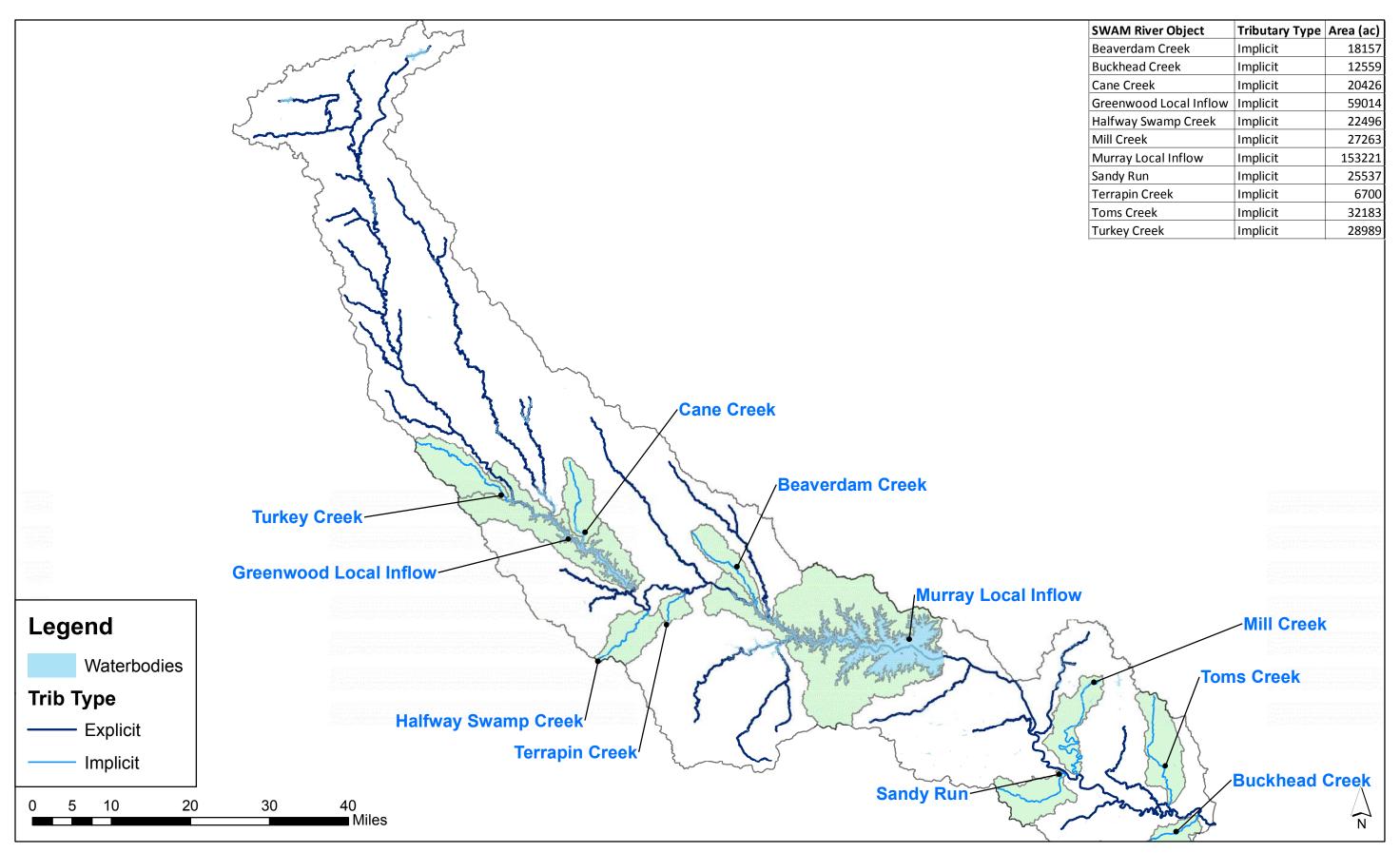




Figure 6-2. Implicit Tributaries in the Saluda River Basin

(represented with implicit tributary objects) for Lake Murray and Lake Greenwood. The inset table provides the corresponding drainage area in acres.

6.1.3 Reach Gains and Losses

In SWAM, reach gain/loss factors capture ungaged flow gains and losses associated with increasing drainage area with distance downstream and/or interaction with subsurface flow (leakage, seepage). Reach gain/loss factors are the primary parameters adjusted during model calibration, as further explained in Section 7. The gain/loss factors are only applied to the input headwater flows and represent a steady and uniform gain/loss volume, proportional to stream length, along the length of the designated reach. The mainstem gain/loss factor is specified on a per unit mile basis. For example, if the mainstem headwater flow is 10 cfs in a given timestep with a gain factor of 0.1 per mile specified for the entire mainstem reach, then the model applies a rate of gain of 1 cfs/mile throughout the length of the mainstem. At the end of a 5 mile reach with no other inflows or outflow, the flow would be 15 cfs. For all other tributary objects, the gain/loss factor is specified as a total subbasin flow gain factor, used to calculate total natural (unimpaired) flow at the end of the designated reach. For example, if a tributary flow is 10 cfs in a given timestep, with a gain factor of 5, then the end-of-reach flow (with no other inflows or outflows) is 50 cfs. The model linearly interpolates when calculating the unimpaired flow at intermediary points in the reach. Both mainstem and tributary flow factors can be spatially variable in the model for up to five (5) different sub-reaches. For further discussion on SWAM reach gain/loss factors, please refer to the SWAM User's Manual.

Tributary gain/loss factors are the primary calibration parameters in the model, as discussed in Section 7. Recognizing the uncertainty in these parameters, factors are adjusted, as appropriate, to achieve a better match of modeled vs. measured downstream flows. As a starting point in the model, however, overall tributary sub-basin gain factors were prescribed in the model based only on the ratios of drainage areas (headwater vs. confluence). Drainage areas are shown in Figures 6-1 and 6-2 and calculated tributary and mainstem subbasin flow factors are summarized in **Table 6-3**.

6.2 Reservoirs

Six reservoirs are represented in the Saluda River Basin Model: Table Rock Reservoir, North Saluda Reservoir, Saluda Lake, Lake Rabon, Lake Greenwood, and Lake Murray. **Table 6-4** provides a summary of model inputs and other information used to characterize each reservoir. Additional details and explanation for certain reservoir inputs are summarized below.

6.2.1 Evaporation

In SWAM, evaporative losses can be specified using monthly-varying seasonal rates (inches per day or percent volume) or with a user-specified timeseries of monthly or daily evaporative losses (inches per month or inches per day). In both the calibration and baseline models, evaporative losses are specified using a timeseries developed during the UIF process. Evaporation was computed using the Hargreaves method from daily temperature data and latitude. Temperature stations for were chosen based on proximity to pan evaporation sites. Temperature stations used in developing evaporative loss estimated are listed in Table 6-4.

6.2.2 Direct Precipitation

Direct precipitation to the surface of Lake Greenwood and Lake Murray was included as part of the local inflow tributary object. Direct precipitation to the other four, much smaller reservoirs was considered insignificant, and not explicitly included in the model. However, precipitation rates were



Table 6-3. Model Tributary Inputs

	Tributary	Confluence	Confluence		Headwater	End	Gain/Loss
SWAM Tributary Object	Type	Stream	Location	Area (ac)	ID	Mile	Factor
	Type	Stream	(mile)		יםו	IVIIIC	(unitless)
Big Beaver Creek	Explicit	Mainstem	204	29,047	SLD31	9	4.6
Big Beaverdam Creek	Explicit	Bush River	16	7,766	SLD218	4	3.8
Big Creek	Explicit	Mainstem	60	12,452	SLD211	11	17.1
Broad Mouth Creek	Explicit	Mainstem	74	21,733	SLD213	14	8.3
Brushy Creek	Explicit	Mainstem	46	23,341	SLD207	11	18.2
						19	1.25
Bush River	Explicit	Mainstem	139	77,215	SLD34	26	2.53
						32	5
Cedar Creek	Explicit	Mainstem	221	103,061	SLD225	9	2.4
ecual Creek	Expirere	Widinstelli		103,001	JLDLLS	24	6.3
Clouds Creek	Explicit	Mainstem	143	71,743	SLD221	1	
0.0000			2.5	, 1,,	015111	28	46
Congaree Creek	Explicit	Mainstem	185	99,229	SLD139	17	4.4
Doddies Creek	Explicit	Mainstem	23	7,207	SLD204	4	7
Georges Creek	Explicit	Mainstem	35	21,123	SLD206	9	9.3
Gills Creek	Explicit	Mainstem	186	47,111	SLD227	7	1.2
			100			15	2.8
Grove Creek	Explicit	Mainstem	57	22,217	SLD08	8	1.8
Hamilton Creek	Explicit	Georges Creek	6	2,581	SLD05	2	2.5
Hurricane Creek	Explicit	Mainstem	50	9,585	SLD210	6	15
Jackson Creek	Explicit	Gills Creek	3	12,291	SLD226	8	23.4
Laurel Creek	Explicit	Reedy River	15	7,449	SLD212	4	3.8
Little River	Explicit	Mainstem	128	147,256	SLD216	30	8.5
Little Mivel	Explicit	Widinstelli	120	147,230	JLDZIO	41	8.7
Little Saluda River	Explicit	Mainstem	142	141,298	SLD129	27	12.2
Ninety-Six Creek	Explicit	Mainstem	110	91,523	SLD17	9	3.7
North Rabon Creek	Explicit	Rabon Creek	3	32,931	SLD15	4	1.4
North Saluda River	Explicit	Mainstem	19	48,294	SLD200	3	6.5
North Salada Mivel	•	Widinstelli	13	10,231	JLDLOO	23	13.1
Oolenoy River	Explicit	South Saluda River	18	31,454	SLD203	12	11.8
Rabon Creek	Explicit	Reedy River	65	81,324	SLD14	4	1
		needy mee.		01,01	01011	18	2.5
Reedy River	Explicit	Mainstem	95	257,031	SLD205	59	19.5
necay mver	Expricit	Widinstelli	33	237,031	JLDL03	69	25
						32	0.05
Mainstem	Explicit	none	none	2,055,737	SLD02	82	0.02
	ZAP.IOIC			_,000,00	01202	117	0
						500	0.01
South Saluda	Explicit	Mainstem	12	66,442	SLD201	2	2
						21	6.7
Twelvemile Creek	Explicit	Mainstem	174	30,029	SLD223	18	11.1
West Creek	Explicit	Clouds Creek	19	15,019	SLD222	8	3.8
Wilson Creek	Explicit	Ninety-Six Creek	7	49,512	SLD215	11	1.5
Beaverdam Creek	Implicit	Mainstem	135	18,157	SLD162	0	1
Broad River	Implicit	Mainstem	176	3,412,232	-	10	0.9
Buckhead Creek	Implicit	Mainstem	227	12,559	SLD167	0	1
Cane Creek	Implicit	Mainstem	99	20,426	SLD160	0	1
Greenwood Local Inflow	Implicit	Mainstem	100	59,014	SLD301	1	0.2
Halfway Swamp Creek	Implicit	Mainstem	113	22,496	SLD161	0	1
Mill Creek	Implicit	Mainstem	197	27,263	SLD165	0	1
Murray Local Inflow	Implicit	Mainstem	149	153,221	SLD302	1	1
Sandy Run	Implicit	Mainstem	195	25,537	SLD164	0	1
Terrapin Creek	Implicit	Mainstem	124	6,700	SLD163	0	1
Toms Creek	Implicit	Cedar Creek	22	32,183	SLD166	0	1
Turkey Creek	Implicit	Mainstem	88	28,989	SLD115	0	1



Table 6-4. Reservoir Inputs

Reservoir	Purpose	Receiving Stream	Temperature Station for Evaporation	Precipitation Station	Release Location (mi)	Storage Capacity (MG)	Initial Storage (MG)	Dead Pool (MG)	Area- Capacity Table	Operating Rules
Table Rock	Water supply	South Saluda River	Clemson W381111	NA	2	8,856	5,000	3,577	Simple	No minimum releases or storage targets
North Saluda	North Saluda Water supply	North Saluda River	Greer W03870	NA	3	23,899	20,000	10,836	Simple	No minimum releases or storage targets
Saluda Lake	Power, water supply, and industry	Mainstem (Saluda)	Clemson 381770	NA	30	2,450	2,000	0	Simple	No minimum releases or storage targets
Lake Rabon	Water supply, flood control Rabon Creek & recreation	Rabon Creek	Union 388786	NA	4	2,946	2,500	0	Simple	No minimum releases or storage targets
Lake Greenwood	Power, recreation, and water supply	Mainstem (Saluda)	Union 388786	USHCN Gage 385017	101	82,760	50,000	10,000	Simple	Minimum release at Buzzards Roost Hydro dependent on season and reservoir inflow ¹ ; Monthly storage targets
Lake Murray	Power, recreation, and water supply	Mainstem (Saluda)	Columbia W13883	USHCN Gage 385200	169	525,881	400,000	319,000 Simple	Simple	Minimum release requirement to maintain 285 cfs at USGS gage 02169000 ² ; Monthly storage targets; Striped Bass Enhancement Flow regime is an optional rule that can be selected ³

through October minimum flow release is 400 cfs when inflow is above 566 cfs; (b) 300 cfs when inflow is between 566 and 466 cfs; (c) 205 cfs when inflow is between 466 and 366 cfs; and (d) 225 cfs or inflow, whichever is less when inflow is below 366 cfs. During calibration, storage targets were set based on the guide curve in effect between 1993 and 2009. Note 1 - For Buzzard's Roost Hydro, during November through June (non-peaking months), the minimum flow release is 400 cfs or the inflow, whichever is smaller. During July

and May; May minimum release = 1,000 cfs; and April minimum release is conditioned on Broad River flow at the confluence of the Saluda. For April, the model attempts to maintain Note 2 - For historical Lake Murray operations (calibration model only), the rules are based on maintaining at least 285 d's at USGS gage 02169000 (just downstream of Twelvemile at least 9,000 cfs in the Congaree just downstream of the Broad confluence, subject to: if Broad River flow (at confluence) is < 2,500 or > 8,000 cfs, then minimum release = 1,000 cfs; Creek), year round. If Twelvemile Creek confluence flow is >= 285 cfs in a given timestep, then there is no Lake Murray release requirement. If Twelve mile Creek flow is < 285 cfs, then the lake minimum release = 285 — Twelvemile Creek flow. During calibration, the "Previous Existing Rule Curve" was used to set target reservoir elevations. The baseline model target Note 3 - For Lake Murray, the release rule associated with the Striped Bass Enhancement Program is incorporated as follows: Minimum release = 700 dfs for all months except April elevations have been set per the new "Target Reservoir Elevation" curve, as contained in the pending FERC license. Lake levels suggest that this curve has been followed since 2009 otherwise minimum release = 9,000 – Broad River flow



factored into the calculation of non-negative net evaporation rates for these smaller reservoirs. In other words, when evaporation was equal to or exceeded precipitation, precipitation was subtracted from the gross evaporation rate to calculate net rates. For timesteps where precipitation exceeded evaporation, net evaporation rates were set to zero.

6.2.3 Area-Capacity Relationships and Flood Control Outflow

Area-capacity relationships for the six reservoirs are summarized in **Table 6-5**. The area-capacity relationships are represented in SWAM with 12 points or less, which in some cases is a simplified representation of the full tabular relationship. No bathymetric or area-capacity information was found for Saluda Lake or Lake Greenwood; therefore, these two reservoirs have area-capacity defined by known empty and full surface areas, and a very simplified linear relationship is assumed.

For Lake Rabon, the area-capacity relationship is derived from the curve provided in as-built drawings. The storage capacity (top of the dam) is much higher than normal pool capacity specified in the model. The dams' spillways pass flood waters, keeping reservoir levels well below the top of the dam. The model includes a normal pool capacity of 2,946 million gallons (MG), but includes a flood control outflow beginning at 90% full, as shown in **Table 6-6**. During calibration, a very small flood control outflow was assigned to Saluda Lake to better reflect observed historical operations. All other reservoirs are not assigned a specific flood control outflow.

Table 6-5. Reservoir Area-Capacity Relationship

Reservoir	Volume (MG)	Area (Acres)
	0	0
	5,619	380
	6,198	398
	6,522	407
Table Rock	7,061	423
	7,622	443
	8,213	460
	8,826	476
	8,856	478
	0	0
	16,022	848
	16,667	865
	17,312	882
	17,957	899
North Saluda	18,663	913
North Saluda	19,261	931
	20,503	970
	21,789	1,003
	22,449	1,018
	23,121	1,033
	23,899	1,052

Reservoir	Volume (MG)	Area (Acres)
Saluda Lake	0	0
Jaiuua Lake	3,000	330
	0	0
	1,629	420
	3,258	600
Lake Rabon	5,213	800
	7,168	1,000
	9,123	1,200
	11,078	1,400
Lake Greenwood	0	0
Lake Greenwood	82,760	11,400
	0	0
	319,000	35,000
	350,000	37,600
	375,000	39,600
	400,000	41,500
Lake Murray	425,000	43,400
	450,000	45,100
	475,000	46,800
	500,000	48,400
	525,000	49,900
	526,000	50,000



Table 6-6. Flood Control Outflow

Reservoir	% Volume	Outflow (cfs)
Table Rock	0	0
Table Nock	100	0
	0	0
North Saluda	45	0
	100	0
Saluda Lake	0	0
Saluua Lake	100	1
	0	0
Lake Rabon	89	0
Lake Rabon	90	12
	100	12
Lake Creenwood	0	0
Lake Greenwood	100	0
Lako Murray	0	0
Lake Murray	100	0

6.2.4 Releases and Operating Rules

Reservoir release locations are assigned in the model based on best available information for dam and outflow locations. Actual modeled releases are calculated in the model based on prescribed operating rules and release targets (see SWAM User's Manual). Of the six Saluda River Basin reservoirs, only Lake Greenwood and Lake Murray have pre-defined operating rules that merit inclusion in the model. These are summarized in Table 6-4. Both reservoirs are operated following a rule curve. The monthly storage targets defined by the rule curves which were input into the baseline model and calibration model are provided in **Table 6-7**. For each reservoir, different rules curves were in effect during the calibration period, compared to the rule curves followed today and incorporated into the baseline model.

6.2.4.1 Lake Greenwood

Lake Greenwood's release rule specifies minimum releases through the dam dependent on season (peak vs. non-peaking months) and reservoir inflow, representing operations of the Buzzards Roost Hydro. In addition to these prescribed release targets, monthly storage targets are prescribed and serve as a second set of considerations for calculating reservoir releases and operations.

6.2.4.2 Lake Murray

For the calibration model, Lake Murray's releases are calculated in the model based on flows at USGS gage 02169000 (SLD26), where a mean daily flow of at least 285 cfs must be maintained. As with Lake Greenwood, monthly storage targets are also included in the model, as secondary considerations in simulated operations. In simulations of future conditions, Lake Murray's releases may include trout and striped bass environmental flow requirements as defined by Instream Flow Incremental Flow



Methodology Study. These requirements have been prescribed in the baseline model as an optional release rule.

Table 6-7. Reservoir Monthly Storage Targets (in Million Gallons)

Reservoir	Model	Jan	Feb	Mar	Apr	May	Jun
Lake Greenwood	calibration	73,657	76,140	79,450	81,105	81,105	81,105
Lake Greenwood	baseline	74,484	75,312	77,795	81,105	81,105	81,105
Lake Murray	calibration	411,048	438,105	481,253	511,836	511,836	496,357
Lake Murray	baseline	432,137	476,357	491,836	515,000	515,000	515,000

Reservoir		Jul	Aug	Sep	Oct	Nov	Dec
Lake Greenwood	calibration	81,105	81,105	77,795	77,795	77,795	77,795
Lake Greenwood	baseline	81,105	81,105	81,105	81,105	79,450	76,140
Lake Murray	calibration	481,253	452,137	428,485	411,048	408,000	408,000
Lake Murray	baseline	491,836	491,836	484,049	476,357	467,250	446,516

6.3 Water Users

6.3.1 Sources of Supply

Table 6-8 summarizes the sources of supply for all Water User objects included in the model. This information includes withdrawal tributaries (or reservoirs), diversion locations, and permit limits. As noted in the table, only several minor differences exist between the calibration and baseline model with respect to water users. Most notably, Duke Power's Lee Steam Station came off-line in late 2014, and therefore it is not included in the baseline model. Several out-of-basin sources are represented as Discharge objects (discussed below) and therefore don't appear in Table 6-8.

6.3.2 Demands

Table 6-9 presents the monthly demand for Municipal (WS), Industrial (IN), Mining (MI), and Thermopower (PT) Water User objects in the baseline model. Monthly irrigation demands for Golf Course (GC) and Agricultural (IR) Water User objects are presented in **Table 6-10**. The baseline model monthly demand assigned to each Water User object was calculated by averaging monthly demands (as reported to DHEC) over the ten-year period from 2004 through 2013. Demands for the calibration period (1983 through 2013) were input as a timeseries of monthly values based on monthly withdrawals reported to DHEC and supplemented by data collected from each water user by CDM Smith.

6.3.3 Transbasin Imports

In South Carolina, there are many examples of water users who access source waters in multiple river basins and/or discharge return flows to multiple basins. In order to consistently represent transbasin imports and exports in the SWAM models, a set of guidelines were developed, which are summarized in **Appendix C – Guidelines for Representing Multi-Basin Water Users in SWAM**. In the Saluda River Basin Model, several water users import water from outside the basin. These include:



Table 6-8. Water User Objects and Sources of Supply Included in the Saluda River Basin Model

Model Object ID	Facility Name	Source of Supply	Intake ID	Diversion Location (mi)	Permit Limit (MGM)	Note
GC: Cliffs Club	Cliffs Club At Valley	North Saluda River	23GC013S01	7	13.4	1
GC: Forest Lake	Forest Lake Club	Gills Creek	40GC002S01	4	11.3	1
GC: Furman U.	Furman University Golf Club	Reedy River	23GC004S01			1
	·	<u>'</u>	23GC004S02	1	26.8	1
GC: Golden Hills	Golden Hills Golf & Country Club	Twelvemile Creek	32GC007S01	1	32.58	1
GC: Lexington	Country Club of Lexington	Twelvemile Creek	32GC004S01		22.3	1
GC: Ponderosa	Ponderosa Country Club	West Creek	32GC010S01	3	44.6	1
GC: Rolling Green	Rolling Green Golf Club	Doddies Creek	39GC002S01			1
GC: Smithfields	Smithfields Country Club	Brushy Creek	39GC003S01	1	-	1
			40GC005S03	1	9.78	1
GC: The Members	The Members Club At Wildewood	Jackson Creek	40GC005S05	1	6.69	1
			40GC005S06	1	3.35	1
GC: The Preserve	The Preserve At Verdae	Laurel Creek	23GC014S01	1	58.03	1
GC: The Rock	The Rock At Jocassee Gc	Oolenoy River	39GC006S01	1	7.14	1
IN: CMC Steel	CMC Steel South Carolina	Congaree River	32IN051S01	181	48.3	1
INI. DAK	DAK (Eastman Chemical/SC	Canada Diver	00111001501	100	F 404	1
IN: DAK	Operations)	Congaree River		+		1
IN: Shaw	Shaw Industries Group Plant 8S	Saluda River			1365	1
IR: Beechwood	Beechwood Farm	North Saluda River			-	1
IR: Bush River Farms	Bush River Farms	Bush River		+	-	1
IR: Frick Farm	Frick Farm	Clouds Creek		+	-	1
ID 11 5	Laulan Farra	Bush River			-	1
IR: LesLea Farms	Leslea Farms	Big Beaverdam Creek		1	-	1
		Bush River			-	1
IR: Mayer Farm	Mayer Farm	Bush River			-	1
					-	11
		Brushy Creek			-	11
IR: Merritt Bros	Merritt Bros Inc				-	11
		Hurricane Creek	40GC002S01	1		
10.0 1 11 5	0 1 1 5 110	2: 2 1 2 1			-	1
IR: Overbridge Farm	Overbridge Farm LLC	Big Beaverdam Creek		+	-	1
IR: Satterwhite Farm	Satterwhite Farms	Bush River		+	-	1
					-	1
ID: Canas Climban	Sana Climban Farman	Turaliza mila Casali			-	1
IR: Sease Clinton	Sease Clinton Farms	Twelvemile Creek			-	1
					-	1
				+	-	1
ID Committee	6	T all and a Const			-	1
IR: Sease James	Sease James R Farms Inc	Twelvemile Creek		J	-	1
				1	-	1
10.61	ļ., , , ,	B: 0 1		+	-	1
IR: Stoneybrook	Stoneybrook	Big Creek			-	1
				+	-	1
IR: Titan Farms	Titan Farms	Clouds Creek		+	-	1
		1 / -:			-	1
		Little Saluda River	41IKU14S08	2	-	1

Note 1 indicates the withdrawal is currently active, and was included in both the baseline and calibration model.

 $Note\ 2\ indicates\ the\ with drawal\ was\ previously\ active,\ and\ was\ included\ in\ the\ calibration\ model.$

Note 3 indicates the withdrawal occurs outside the Saluda Basin.



^{*} Indicates the maximum transfer amount

Table 6-8. Water User Objects and Sources of Supply Included in the Saluda River Basin Model (continued)

Model Object ID	Facility Name	Source of Supply	Intake ID	Diversion Location (mi)	Permit Limit (MGM)	Note
IR: Twin Oaks Farm	Twin Oaks Farm	Hurricane Creek	04IR001S01	3	-	1
IR: Walker Farm	Walker Farm	Cedar Creek	40IR001S01	1	-	1
IR: Watson Jerrold Farm	Watson Jerrold & Sons	Clouds Creek	02IR011S09	1	-	1
IR: Watson Joe Farm	Watson Joe Farm	Clouds Creek	41IR004S01 41IR004S02	3	-	1
	Martin Marietta Materials - Cayce					
MI: Martin Marietta	Quarry	Congaree River	32MI001S01	180	66.96	1
MI: Vulcan	Vulcan Materials	Saluda River	04MI001S01	40	16	1
PT: Duke Lee Station	Duke Energy Carolinas LLC	Saluda River	04PT001S01	58	-	2
PT: SCE&G	SCE&G - McMeekin Station	Saluda River/Lake Murray		169	5175	1
WS: Belton Honea	Belton-Honea Path WTP	Saluda River	04WS005S01	65	124	1
WS: Cayce	City Of Cayce WTP	Congaree River	32WS004S02	182	722.3	1
WS: Columbia	City of Columbia - Lake Murray Water Plant	Saluda River/Lake Murray	40WS002S02	169	3875	1
	City of Columbia - Canal Water Plant	Out of basin (Broad)	40WS054S01	999	3875	1,3
	Easley Combined Utilities - D.L.					
WS: Easley	Moore WTP	Saluda River	39WS001S01	30	1116	1
		North Saluda River/North				
		Saluda Res	23WS002S01	3	1860	1
MC. Carrierilla	Canada illa Mataril D. Stavall Blant	South Saluda River/Table				
WS: Greenville	Greenville Water L.B. Stovall Plant	Rock Res	23WS002S02	2	1085	1
		South Saluda River/Table				
		Rock Res	23WS002S03	2	992	2
	C:	Saluda River/Lake	2 11 15 20 4 50 4	00	050.5	
WS: Greenwood	City of Greenwood (Wise Plant)	Greenwood	24WS001S01	99	852.5	1
		Saluda River/Lake				
WS: Greenwood	City of Greenwood (Wise Plant)	Greenwood	24WS001S02	99	852.5	1
		Little River	30WS002S01	1	-	2
WS: Laurens	Laurens WTP	Rabon Creek	30WS002S02	4	911	1
		Rabon Creek/Lake Rabon	30WS002S03	6	1106	1
WS: NCWSA	NCWSA - Lake Murray WTP	Saluda River/Lake Murray	36WS002S01	169	186	1
WS: Newberry	City Of Newberry WTP	Saluda River	36WS001S01	129	682	1
,	,					
WS: SCWSA	SCWSA - Raw Water Intake	Saluda River/Lake Murray		169	465	1
MC Mari Cal arti	West Call subject AFT	Saluda River	32WS008S01	177	NA	1
WS: West Columbia	West Columbia WTP	Saluda River/Lake Murray	32WS052S01	169	NA	1
WS: Williamston	Town of Williamston	Big Creek	04WS011S01	5	-	2

Note 1 indicates the withdrawal is currently active, and was included in both the baseline and calibration model.

 $Note\ 2\ indicates\ the\ with drawal\ was\ previously\ active,\ and\ was\ included\ only\ in\ the\ calibration\ model.$

Note 3 indicates the withdrawal occurs outside the Saluda Basin.



 $[\]hbox{* Indicates the maximum transfer amount}$

Table 6-9. Baseline Model Average Monthly Demand for WS, IN, MI, and PT Water Users

			Baselir	ne Model Av	erage Month	lly Demand (MGD)		
Month	IN: CMC Steel	IN: DAK	IN: Shaw	MI: Martin Marietta	MI: Vulcan	PT: SCE&G	WS: Belton Honea	WS: Cayce	WS: Columbia
Jan	0.2	55.2	21.5	0.9	0.0	140.1	1.9	2.7	51.8
Feb	0.2	56.7	20.9	0.8	0.0	122.6	1.9	2.8	51.4
Mar	0.1	57.7	20.8	0.8	0.0	114.3	1.8	2.9	53.7
Apr	0.1	60.2	22.0	1.0	0.0	88.9	1.9	3.1	61.1
May	0.2	66.3	23.5	0.7	0.0	125.8	2.2	3.4	68.1
Jun	0.2	74.0	25.0	0.8	0.0	150.7	2.4	3.5	72.1
Jul	0.2	78.1	26.8	0.8	0.5	155.5	2.2	3.5	75.1
Aug	0.1	76.1	27.8	0.9	0.4	149.0	2.3	3.6	73.0
Sep	0.2	70.1	26.8	0.9	0.0	109.2	2.2	3.4	69.3
Oct	0.2	61.7	25.0	0.9	0.0	99.4	2.0	3.2	63.4
Nov	0.2	55.1	23.0	0.7	0.0	117.1	2.0	3.0	57.5
Dec	0.2	52.3	22.0	0.6	0.0	128.9	1.9	2.8	51.9

			Baseline Mod	del Average I	Monthly Den	nand (MGD)		
N.A. a. a. b.	WC. Faalan	WS:	WS:	WS:	WS:	WS:	WS: W.	WS:
Month	WS: Easley	Greenville	Greenwood	Laurens	NCWSA	Newberry	Columbia	Columbia
Jan	7.1	25.6	9.6	2.1	0.7	1.1	9.4	47.7
Feb	6.8	25.9	9.7	2.1	0.7	0.9	9.5	47.1
Mar	6.9	27.4	9.6	2.1	0.7	0.8	9.9	50.5
Apr	7.7	32.0	10.3	2.3	0.8	1.3	12.0	58.7
May	8.8	37.8	11.1	2.2	0.8	1.8	13.9	64.4
Jun	9.3	41.9	11.7	2.3	0.9	1.9	14.7	68.3
Jul	9.5	43.5	12.0	2.4	0.9	2.0	14.6	71.6
Aug	9.4	42.8	11.9	2.5	0.8	2.0	14.0	71.2
Sep	8.7	40.0	11.1	2.5	0.8	1.8	13.5	66.2
Oct	8.2	35.0	10.6	2.3	0.8	1.6	11.6	61.2
Nov	7.4	29.9	10.0	2.2	0.7	1.4	10.3	54.3
Dec	7.1	24.9	9.4	2.2	0.7	1.0	9.7	46.8



Table 6-10. Baseline Model Average Monthly Demand for GC and IR Water Users

	Baseline Model Average Monthly Demand (MGD)											
Month	GC: Cliffs Club	GC: Forest Lake	GC: Furman	GC: Golden Hills	GC: Lexington	GC: Ponderosa	GC: Rolling Green	GC: Smithfield CC	GC: The Members	GC: The Preserve	GC: The Rock	
Jan	0.01	0.03	0.00	0.00	0.01	0.03	0.00	0.01	0.01	0.01	0.01	
Feb	0.01	0.01	0.00	0.01	0.00	0.04	0.01	0.04	0.01	0.01	0.01	
Mar	0.05	0.05	0.01	0.01	0.02	0.06	0.11	0.13	0.02	0.02	0.02	
Apr	0.10	0.14	0.02	0.03	0.10	0.07	0.10	0.17	0.06	0.06	0.04	
May	0.10	0.14	0.03	0.03	0.13	0.10	0.16	0.21	0.16	0.16	0.07	
Jun	0.15	0.15	0.05	0.04	0.11	0.10	0.20	0.25	0.19	0.19	0.06	
Jul	0.14	0.20	0.06	0.05	0.12	0.11	0.12	0.28	0.21	0.21	0.07	
Aug	0.10	0.14	0.07	0.05	0.11	0.10	0.13	0.25	0.24	0.24	0.07	
Sep	0.14	0.18	0.07	0.05	0.12	0.09	0.14	0.17	0.17	0.16	0.06	
Oct	0.09	0.10	0.04	0.04	0.08	0.06	0.08	0.17	0.09	0.08	0.04	
Nov	0.05	0.07	0.01	0.03	0.03	0.05	0.02	0.02	0.04	0.03	0.02	
Dec	0.01	0.03	0.00	0.01	0.00	0.02	0.00	0.02	0.01	0.01	0.01	

				Baseline I	Model Ave	rage Mon	thly Dema	nd (MGD)			
Month	IR: Beechwood	IR: Bush River Farms	IR: Frick Farm	IR: LesLea Farms	IR: Mayer Farm	IR: Merrit Bros	IR: Overbridge Farm	IR: Satterwhite Farm	IR: Sease Clinton	IR: Sease James	IR: Stonybrook
Jan	0.00	0.02	0.00	0.00	0.00	0.00	0.00	0.02	0.02	0.08	0.00
Feb	0.00	0.13	0.00	0.00	0.00	0.00	0.00	0.02	0.04	0.15	0.01
Mar	0.03	0.41	0.00	0.00	0.05	0.00	0.00	0.02	0.06	0.27	0.01
Apr	0.13	0.51	0.00	0.01	0.05	0.00	0.02	0.02	0.08	0.54	0.01
May	0.13	0.52	0.37	0.04	0.05	0.02	0.02	0.03	0.12	0.66	0.01
Jun	0.17	0.54	0.90	0.12	0.05	0.04	0.02	0.05	0.18	0.69	0.02
Jul	0.29	0.53	0.73	0.15	0.04	0.07	0.01	0.04	0.25	0.69	0.02
Aug	0.29	0.53	0.48	0.07	0.02	0.07	0.05	0.04	0.23	0.79	0.03
Sep	0.31	0.55	0.16	0.07	0.02	0.04	0.02	0.03	0.21	0.82	0.02
Oct	0.13	0.52	0.07	0.00	0.03	0.01	0.00	0.02	0.13	0.78	0.00
Nov	0.03	0.08	0.00	0.00	0.03	0.00	0.00	0.02	0.09	0.50	0.00
Dec	0.00	0.04	0.00	0.00	0.00	0.00	0.00	0.02	0.04	0.22	0.00

	Baseline I	Model Ave	rage Mon	thly Dema	nd (MGD)
Month	IR: Titan Farms	IR: Twin Oaks Farm	IR: Walker Farm	IR: Watson Jerrold	IR: Watson Joe Farm
Jan	0.04	0.00	0.00	0.29	0.00
Feb	0.04	0.00	0.00	0.34	0.00
Mar	0.50	0.00	0.00	0.47	0.00
Apr	0.98	0.01	0.00	0.64	0.10
May	1.38	0.01	0.00	1.19	0.16
Jun	1.71	0.02	0.01	1.51	0.19
Jul	1.60	0.03	0.00	1.82	0.19
Aug	1.08	0.02	0.00	1.78	0.20
Sep	0.95	0.02	0.00	1.72	0.20
Oct	0.33	0.00	0.00	1.27	0.14
Nov	0.01	0.00	0.00	0.81	0.03
Dec	0.01	0.00	0.00	0.43	0.00



- The City of Columbia (**WS: Columbia**) imports water from the Broad River Canal Plant located in the Broad River Basin, in addition to its withdrawal in the Saluda Basin on Lake Murray. In both the calibration and baseline models, the import of water from the Broad is treated as a transbasin import in SWAM, and is recognized as Source Water Account #2.
- In 1997, Williamston stopped withdrawing from Big Creek and began importing water from the Savannah River Basin. In the calibration model, for simplicity, the Big Creek withdrawal is included as the source over the entire calibration period; but the demand is dropped to zero after 1997. In the baseline model, Williamston is represented as a Discharge object (Williamston Import), reflecting the fact that its only source comes from outside the Saluda River Basin, but return flows discharge inside the basin. In the calibration model, Williamston is represented as a Water User object (WS: Williamston), since it had an in-basin source until 1997.
- The City of Clinton is represented as a Discharge object (**Clinton Import**), as its water is sourced exclusively from the Broad River Basin, with return flow discharges to the Saluda River Basin.
- Greenville Water System, which serves the City of Greenville and provides water to other, nearby systems, has three sources of surface water. Two sources, Table Rock Reservoir and North Saluda Reservoir, are located in the Saluda River Basin. The third source is Lake Keowee located in the Savannah River Basin. Consistent with the guidelines, the WS: Greenville Water User object accounts for water sourced only in the Saluda. Water sourced from the Savannah is considered a secondary supply and is represented by three Discharge objects, Greenville Import/Georges, Greenville Import/Mauldin, and Greenville Import/Reedy. In the Savannah River Basin Model, a WS: Greenville Export Water User object will represent the Lake Keowee withdrawal.

The monthly demand associated with the City of Columbia's Broad River Canal withdrawal is presented in **Table 6-11**. As noted above, all other transbasin imports are treated as discharges, and are represented by Discharge objects.

Table 6-11. Baseline Model Average Monthly Transbasin Imports

	BaselineModel Avg Monthly Transbasin Import (MGD)
Month	WS: Columbia
Jan	30.9
Feb	30.4
Mar	32.6
Apr	37.9
May	41.6
Jun	44.1
Jul	46.3
Aug	46.0
Sep	42.8
Oct	39.5
Nov	35.1
Dec	30.2



6.3.4 Consumptive Use and Return Flows

As discussed in Section 4.2, return flows (discharges) can be simulated two ways in SWAM. They can be associated with a Water User object or specified within a Discharge object. **Table 6-12** summarizes the calibration and baseline model objects representing return flows, their location, and the percent of return flow assigned to each location. In this table, the "% of Return Flow" represents the allocation to one or more discharge locations, not the consumptive use percentage. In many instances, multiple NPDES discharge locations associated with a unique Water User object were lumped together, based on their close proximity to one another (e.g., Duke's four Lee Steam Station Discharges were lumped together in the calibration model). No returns are assumed for golf course and agricultural irrigation (i.e., 100% consumptive use).

Table 6-13 presents the monthly percent consumptive use for water users with known return flows. For all municipal and industrial water users, consumptive use was calculated from DHEC-reported withdrawals and discharges over the baseline period (2004 through 2013). The two mines, Vulcan and Martin Marietta, have general use discharge permits, which have flows that do not require reporting to DHEC. Instead, returns for these two water users is defined by the estimated percent of return flow indicated in their surface water withdrawal permits. For SCE&G McMeekin Station, NPDES records of discharges were inconsistent and incomplete, therefore a representative consumptive use for thermoelectric facilities of 26% was assumed based on estimates provided by the facility. For the Duke power station (calibration model only), an assumed consumptive use value of 2.5% is used based on literature (Torcellini, 2003).

Table 6-14 presents the baseline model monthly average returns represented by a Discharge object. The returns were calculated by averaging the DHEC-reported discharges for the baseline period (2004 through 2013).

6.4 Summary

This section has presented the form and numerical values of data that are input into the Saluda River Basin Model, in the context of the model framework discussed in Section 4. Data descriptions are organized according to the model objects which house the data. For more details on SWAM model input requirements and mechanics, readers are referred to the SWAM User's Manual. Note that, as discussed in Section 7, a small portion of these input data may be adjusted as part of the calibration process. For the Saluda River Basin model, these calibration inputs only include reach hydrologic gain/loss factors and, to a very limited extent, reservoir operating rule targets.



Table 6-12. Returns and Associated Model Objects

						% of
			Associated Water		Model	Return
Model Object ID	Facility Name	NPDES Pipe ID	Permit	Discharge Tributary	River Mile	Flow
Returns Represented V	Vithin Water User Objects	<u> </u>	<u> </u>	1	1	
		SC0001333-001			191	100
		SC0001333-01A			191	100
IN: DAK	Eastman Chemical/SC Operations	SC0001333-01D	09IN001	Congaree River	191	100
	Zasaman enemican, se e peranens	SC0001333-01E	05001	conguice miver	191	100
		SC0001333-01F			191	100
		SC0001333-01G			191	100
		SC0003557-001			172	100
IN: Shaw Industries	Shaw Industries Group/Columbia	SC0003557-002	32IN006	Congaree River	172	100
		SC0003557-003			172	100
MI: Martin Marietta	Martin Marietta/Cayce Quarry	SCG730263-000	32MI001	Congaree River	181	100
MI: Vulcan	Vulcan Const Mat/Lakeside	SCG730245-000	04MI001	Saluda River	41	100
		SC0002291-001			59	100
PT: Lee Steam Sta*	Duka Francy/Lag Staam Station	SC0002291-002	04PT001	Saluda River	59	100
PT. Lee Steam Sta	Duke Energy/Lee Steam Station	SC0002291-003	0421001	Saluua River	59	100
		SC0002291-004	1		59	100
		SC0002046-001			171	100
		SC0002046-002	1		171	100
PT: SCE&G	SCE&G/McMeekin Steam Station	SC0002046-003	32PT001	Saluda River	171	100
		SC0002046-004	†		171	100
		SC0002046-005	†		171	100
	Due West WWTF	SC0022403-001	04WS005	Out of basin (Savannah)	1002	6
		SC0045896-001		Saluda River	62	21
WS: Belton Honea	Belton/Ducworth (Saluda)	SC0045896-002	04WS005		1	52
		SC0045896-003	1	Broad Mouth Creek	1	52
WS: Belton Honea/		3600-3630-003			1	32
WS: Greenwood	Ware Shoals/Dairy Street	SC0020214-001	04WS005/24WS001	Saluda River	81/81.5	21/11
WS: Cayce/WS: W.	Wate Shoals/ Daily Street	300020214-001	32WS004/32WS008/	Jaiuua Nivei	81/81.3	21/11
Columbia	Cayce WWTF	SC0024147-001	32WS052	Congaree River	183/184	100/86
Corumbia	Alpine Utilities/Stoop Creek	SC0029483-001	32VV3032	Saluda River	173	6
	East Rich CO PSD/Gills Creek	SC0029483-001 SC0038865-001	+		183	85
WS: Columbia	•		40WS002	Congaree River		9
	Chapin, Town of Richland Co/Broad River WWTF	SC0040631-001	1	Out of basin (Broad)	1005 1005	9
MC. Columbia /MC. M	·	SC0046621-001	40/4/5003/33/4/5009/		1005	9
WS: Columbia/WS: W.		CC0020040 001	40WS002/32WS008/	Canada Divar	102 5/104	05/00
Columbia	Columbia/Metro Plant	SC0020940-001	32WS052	Congaree River	182.5/184	85/86
MC. Feeler	Easley/Golden Creek Lagoon	SC0023035-001	2014/0001	Out of basin (Savannah)	1003	5
WS: Easley	Easley/Georges Creek Lagoon	SC0023043-001	39WS001	Georges Creek	1	15
	Easley/Middle Branch WWTP	SC0039853-001	2011/2002	Brushy Creek	3	80
	WCRSA/Marietta WWTP	SC0026883-001	23WS002	North Saluda River	15	6
	WCRSA/Pelham WWTF	SC0033804-001	2011/2007		1001	48
	WCRSA/Durbin Creek	SC0040002-001	23WS007	Out of basin (Broad)	1001	48
	WCRSA/Gilder Creek	SC0040525-001			1001	48
WS: Greenville	WCRSA/Mauldin Road	SC0041211-001	23WS002	Reedy River	12	39
						l
	WCRSA/Piedmont Regional WWTP	SC0048470-001	23WS002	Saluda River	49	7
	Greenville/N Saluda & Table Rock					
	WTP	SCG646033	23WS002	North Saluda River	15	6
	Greenwood/Wilson Creek WWTF	SC0021709-001		Wilson Creek	1	76
WS: Greenwood	Greenwood/West Alexander	SC0022870-001	24WS001			
. Gicchwood	WWTF			Out of basin (Savannah)	1009	6
	Ninety Six WWTF	SC0036048-001		Ninety Six Creek	81.5	11
		SC0020702-001	30WS002	Little River	1	100
WC-Laurons	Laurens Comm of PW/Laurens			Little River		100
WS: Laurens	Laurens WTP	SCG646028	30VV3002		1	100
WS: Laurens		†	30W3002	Little Saluda River	8	
WS: Laurens WS: Newberry	Laurens WTP	SCG646028	36WS001	Little Saluda River Bush River		100 15 71
	Laurens WTP Saluda, Town of	SCG646028 SC0022381-001			8	15 71
	Laurens WTP Saluda, Town of Newberry/Bush River WWTF	SCG646028 SC0022381-001 SC0024490-001		Bush River	8 20	15 71



Table 6-12. Returns and Associated Model Objects (continued)

			Associated Water		Model	% of Return
Model Object ID	Facility Name	NPDES Pipe ID	Permit	Discharge Tributary	River Mile	Flow
Transbasin Imports Rep	presented by Discharge Objects					
Clinton Import (Broad)	Laurens CO W&S/Clinton-Joanna	SC0037974-001	30WS001	Bush River	10	
Greenville Import/						
Reedy (Savannah)	WCRSA/Lower Reedy River Plant	SC0024261-001	23WS007	Reedy River	24	
Greenville						
Import/Georges						
(Savannah)	WCRSA/Georges Creek	SC0047309-001	23WS007	Saluda River	36	
Greenville						
Import/Mauldin						
(Savannah)	WCRSA/Mauldin Road	SC0041211-001	23WS007	Reedy River	12	
Williamston Import/						
Savannah**	Williamston/Big Creek East WWTP	SC0046841-001	04WS006	Big Creek	11	
In-basin Returns Repres	sented by Individual or Aggregated D	Discharge Objects				
Milliken	Milliken/Gayley Plant	SC0003191-001	none	South Saluda River	19	
Willinkell	Willikelly dayley Flant	SC0003191-T11	none	Jodin Jaidda Niver	19	
	CWS/Watergate Development	SC0027162-001		Twelvemile Creek	175	
	Woodland Hills West SD	SC0029475-001			175	
Agg Discharge 1	Bush River Utilities	SC0032743-001	none	Saluda River	175	
	CWS/I-20 Regional	SC0035564-001		Saluua Kivei	175	
	CWS/Friarsgate SD	SC0036137-001			175	
Agg Discharge 2	Westinghouse Elec LLC/Columbia	SC0001848-001	none	Congaree River	195	
Agg Discharge 2 Devro	Devro Inc/Coria Division	SC0033367-001	none	Congalee Niver	195	
Air Products	Air Products & Chemicals, Inc	SC0048429-001	none	Saluda River	43	
Ing Rand	Ingersoll Rand/G.W. Recovery Sys	SC0048534-001	none	Little River	10	

Note: Returns outside of the Saluda River Basin are indicated in **bold**.

Table 6-13. Baseline Model Monthly Consumptive Use Percentage

		Monthly Consumptive Use (%)												
Month	IN: CMC Steel	IN: DAK	IN: Shaw	MI: Martin Marietta	MI: Vulcan	PT: SCE&G	WS: Belton Honea	WS: Cayce						
Jan	5.0	1.0	3.0	50.0	90.0	2.5	18.0	16.2						
Feb	5.0	1.0	3.0	50.0	90.0	2.5	20.8	13.0						
Mar	5.0	1.0	3.0	50.0	90.0	2.5	16.2	15.6						
Apr	5.0	1.0	3.0	50.0	90.0	2.5	25.2	26.5						
May	5.0	1.0	3.0	50.0	90.0	2.5	37.4	35.3						
Jun	5.0	1.0	3.0	50.0	90.0	2.5	39.6	36.5						
Jul	5.0	1.0	3.0	50.0	90.0	2.5	44.0	36.0						
Aug	5.0	1.0	3.0	50.0	90.0	2.5	46.2	32.3						
Sep	5.0	1.0	3.0	50.0	90.0	2.5	49.1	32.9						
Oct	5.0	1.0	3.0	50.0	90.0	2.5	43.4	28.9						
Nov	5.0	1.0	3.0	50.0	90.0	2.5	38.4	24.7						
Dec	5.0	1.0	3.0	50.0	90.0	2.5	23.8	17.5						



^{*} Only represented in the calibration model (came off-line in 2014). ** Represented by a Water User object in the calibration model and a Discharge object in the baseline model.

Table 6-13. Baseline Model Monthly Consumptive Use Percentage (continued)

		Monthly Consumptive Use (%)											
Month	WS: Columbia	WS: Easley	WS: Greenville	WS: Greenwood	WS: Laurens	WS: NCWSA	WS: Newberry	WS: W. Columbia					
Jan	8.9	61.3	23.5	5.2	20.5	54.5	23.7	30.0					
Feb	6.7	61.8	24.6	5.5	23.9	51.3	18.1	30.0					
Mar	6.1	61.5	28.3	6.6	25.0	54.0	20.7	30.0					
Apr	19.2	68.4	40.4	14.2	29.4	58.7	25.6	30.0					
May	31.1	72.7	48.4	23.0	33.0	63.1	33.4	30.0					
Jun	34.0	71.9	52.8	29.4	32.6	63.2	34.5	30.0					
Jul	36.8	72.7	57.4	29.7	28.4	64.9	36.4	30.0					
Aug	29.5	73.9	54.3	28.2	22.7	68.9	36.0	30.0					
Sep	28.6	71.9	54.4	26.5	16.3	70.2	34.1	30.0					
Oct	26.7	72.2	48.7	22.2	18.4	66.8	32.3	30.0					
Nov	19.3	67.6	37.9	16.7	11.4	63.7	28.7	30.0					
Dec	10.8	61.1	23.1	10.8	13.8	57.7	21.6	30.0					

Table 6-14. Baseline Model Monthly Return Flows for Discharge Objects

					Monthly R	eturn Flow	(MGD)			
Month	Agg Disch 1	Agg Disch 2	Air Products	Clinton Import (Broad)	Williamston Import (Savannah)	Greenville Import/ Georges (Savannah)	Greenville Import/ Mauldin (Savannah)	Greenville Import/ Reedy (Savannah)	Ing Rand	Milliken
Jan	2.81	0.55	0.29	2.37	0.58	1.95	10.23	8.36	4.89	1.23
Feb	3.05	0.56	0.14	2.37	0.62	1.83	11.11	8.38	5.95	1.26
Mar	3.04	0.54	0.20	2.52	0.60	1.92	9.46	8.49	6.36	1.13
Apr	2.82	0.55	0.15	2.21	0.57	1.73	6.40	7.99	6.14	1.37
May	2.57	0.52	0.14	1.97	0.60	1.68	8.04	7.92	6.15	1.39
Jun	2.60	0.55	0.24	1.95	0.66	1.72	10.45	7.33	6.14	1.22
Jul	2.70	0.54	0.15	1.92	0.64	1.66	8.97	7.35	5.93	1.39
Aug	2.88	0.55	0.19	2.05	0.58	1.62	10.06	8.10	5.75	1.35
Sep	2.63	0.54	0.24	1.90	0.53	1.60	5.50	8.14	5.15	1.45
Oct	2.47	0.51	0.14	1.87	0.51	1.51	8.80	7.95	5.59	1.50
Nov	2.52	0.56	0.13	1.88	0.44	1.64	8.81	8.01	4.91	1.45
Dec	2.93	0.56	0.30	2.24	0.59	1.94	8.43	7.65	4.97	1.18



Model Calibration/Verification

7.1 Philosophy and Objectives

SWAM is a water allocation model that moves simulated water from upstream to downstream, combines flows at confluence points, routes water through reservoirs, and allocates water to a series of water user nodes. It is designed for applications at a river basin scale. In common with all water allocation models, neither rainfall-runoff, nor reach routing, are performed in SWAM. As such, the "calibration" process should be viewed differently compared to catchment or river hydrologic modeling.

The overriding objective of the SWAM calibration process is to verify that the model is generally accurately representing water availability in the basin; i.e. that ungaged flow estimates are roughly accurate, that flows are being combined correctly, and that basin operations and water use are well captured. More specifically, the objectives include:

- extending the hydrologic input drivers of the model (headwater unimpaired flows) spatially
 downstream to adequately represent the unimpaired hydrology of the entire basin by
 incorporating hydrologic gains and losses below the headwaters;
- refining, as necessary and appropriate, a small number of other model parameter estimates within appropriate ranges of uncertainty, potentially including: reservoir operational rules, consumptive use percentages, and nonpoint (outdoor use) return flow locations; and
- gaining confidence in the model as a predictive tool by demonstrating its ability to adequately replicate past hydrologic conditions, operations, and water use.

In many ways, the exercise described here is more about model verification than true model calibration. The model parameterization is supported by a large set of known information and data – including tributary flows, drainage areas, water use and return data, and reservoir operating rules. These primary inputs are not changed during model calibration. In fact, only a small number of parameters are modified as part of this process. This is a key difference compared to hydrologic model calibration exercises, where a large number of parameters can be adjusted to achieve a desired modeled vs. measured fit. Because SWAM is a data-driven model and not a parametric reproduction of the physics that govern streamflow dynamics, care is taken so that observed data used to create model inputs are not altered. In calibrating SWAM, generally the primary parameters adjusted are reach gain/loss factors for select tributary objects. These factors capture ungaged flow gains associated with increasing drainage area with distance downstream. Flow gains through a sub-basin are initially assumed to be linearly proportional to drainage area, in line with common ungaged flow estimation techniques. However, there is significant uncertainty in this assumption and it is therefore appropriate to adjust these factors, within a small range, as part of the model calibration process. These are often the only parameters changed in the model during calibration, though adjustments can also be made if needed to reservoir operating rules, consumptive use rates, and flow estimates in ungaged headwater basins. It is important to note that reservoir operating rules are simulated in the verification of the model in lieu of actual historic data on reservoir usage (which is built into the UIF



datasets). This is to help ensure that the model has predictive strength for simulating the continuation of prescribed rules into the future, by demonstrating that the rules adequately reproduce historic reservoir dynamics.

Consideration also needs to be given to the accuracy of the measured or reported data that serve as key inputs to the model and are not adjusted as part of the calibration exercise. For example, historical water withdrawals are reported to DHEC by individual water users based on imperfect measurement or estimation techniques. Even larger errors may exist in the USGS flow gage data used to characterize headwater flows in the model. These errors are known to be upwards of 20% at some gages and under some conditions (USGS, http://wdr.water.usgs.gov/current/documentation.html). The uncertainty of model inputs merits consideration in the evaluation of model output accuracy.

Lastly, in considering the model calibration and verification, it is also important to keep in mind the ultimate objectives of the models. The final models are intended to support planning and permitting decision making. Planners will use the models to quantify impacts of future demand increases on water availability. For example, if basin municipal demands increase by 50%, how will that generally impact river flows and is there enough water to sustain that growth? Planners might also use the models to analyze alternative solutions to meeting projected growth, such as conservation, reservoir enlargement projects, and transbasin imports. With respect to permitting, regulators will look to the model to identify any potential water availability problems with new permit requests and to quantify the impacts of new or modified permits on downstream river flows. In other words, they will look to the model to answer the question of: if a new permit is granted, how will it impact downstream critical river flows and downstream existing users?

Given the methods and objectives described above, there is no expectation that downstream gaged flows, on a monthly or daily basis, will be replicated exactly. The lack of reach routing, in particular, limits the accuracy of the models at a daily timestep. Rather, the questions are only whether the representation of downstream flows is adequate for the model's intended purposes, key dynamics and operations of the river basin are generally captured (as measured by the frequency of various flow thresholds and reasonable representation of the timing and magnitude of the rise and fall of hydrographs), and whether the models will ultimately be useful as supporting tools for the State.

7.2 Methods

For the model calibration exercise, the fully constructed and parameterized Saluda Basin model, as described in Sections 5 and 6, was used to simulate the 1983 to 2013 historical period. As described in these sections, the calibration model includes input data representative of past conditions, rather than current conditions in the basin. The specific simulation time period was selected because of a higher confidence in reported withdrawal and discharge data for this period compared to earlier periods. The 31 year record also provides a good range of hydrologic and climate variability in the basin to adequately test the model, including extended high and low flow periods.

Guided by the principles described in Section 7.1, the following specific steps were followed (in order) as part of the calibration/verification process:

- 1. Tributary headwater flows were extended to the tributary confluence points using drainage area ratios to calculate tributary object subbasin flow factors (see Section 6).
- 2. New implicit tributary objects were added, as needed and based on visual inspection of GIS mapping, to capture ungaged drainage areas and tributary inputs not included in the original



model framework. Note that a list of implicit tributaries included in the Saluda basin model is provided in Section 6.

- 3. Intermediary subbasin flow factors were adjusted for tributary objects to achieve adequate modeled vs. measured comparisons at selected tributary gage targets, based on monthly timestep modeling.
- 4. Mainstem reach gain/loss factors (per unit length) were adjusted to better achieve calibration at mainstem gage locations, based on monthly timestep modeling. This factor can be varied in multiple locations along the main stem.
- 5. Simulated reservoir operating rules were reviewed based on monthly reservoir level modeled vs. measured comparisons. Note that as a result of this review, specific monthly storage targets for Lake Murray were modified slightly from original estimates.
- 6. The adequacy of the daily timestep model was verified by reviewing daily output once the monthly model was calibrated.

All USGS flow gages at downstream locations in the basin with reasonable records within the targeted simulation period were used to assess model performance and guide the model calibration steps described above. These gages are summarized in **Table 7-1**. Note that in order to minimize the uncertainty in our calibration targets, only gaged (i.e. measured) flow records were used to assess model performance as part of this exercise. No ungaged flow estimates or record filling techniques were used to supplement this data set (although many of the input flows were developed through various record extensions techniques). Note also that all upstream basin water use and operations are implicitly represented in these gaged data, thereby providing an ideal target to which the combination of estimated UIFs and historic water uses could be compared. In addition to the flow gages, reported historical reservoir levels (where available) were also used as calibration/verification targets.

Additionally, as described in Section 6, operational storage targets at Lake Murray are known to have changed in late 2002. This change is not represented in the calibration model, which assumes consistent operational rules throughout the simulation. Therefore, to avoid calibration bias at the two flow gages downstream of the lake (SLD25 and SLD27), the period November 2002 through December 2013 was excluded from the calibration analysis for Lake Murray and these two flow gage sites. Two short periods of known reservoir construction and dewatering, in 1990 and 1996, respectively, were also excluded from the analysis for these sites.

Lastly, all water users in the model were checked to ensure that historical demands were being fully met in the model or, alternatively, if demands were not being met during certain periods, that there was a sensible explanation for the modeled shortfalls.

As indicated above, options for model calibration parameters (i.e. those that are adjusted to achieve better modeled vs. measured matches) are limited to a very small group of inputs with relatively high associated uncertainty. In general, and for future basin models, these might include any of the following: mainstem and/or tributary reach hydrologic gain/loss factors, reservoir operational rules, assumed consumptive use percentages, and return flow locations and/or lag times associated with outdoor use. However, the primary calibration parameters in SWAM are the reach gain/loss factors. Adjustments to other parameters are secondary and often not required. For the Saluda basin model calibration, only reach gain/loss factors, and to a very limited extent Lake Murray storage



targets, were adjusted as part of the calibration process. The final model reach gains/losses are presented in Section 6, Table 6-3.

A number of performance metrics were used to assess the model's ability to reproduce past basin hydrology and operations. These include: monthly and daily water user supply delivery and/or shortfalls, monthly and daily timeseries plots of both river flow and reservoir levels, annual and monthly mean flow values, monthly and daily percentile plots of river flow values, annual 7-day low flows with a 10 year recurrence interval (7Q10), and mean flow values averaged over the entire period of record.

Table 7-1. USGS Streamflow Gages Used in Calibration

Project Gage ID	USGS Number	Tributary Object	Periods of Record	Basin Area (sq. mi.)	River Mile
SLD04	02162500	Mainstem	01/1942 - 10/1978 02/1990 - current	295	32
SLD09	02163500	Mainstem	03/1939 - current	580	82
SLD12	02165000	Reedy River	04/1939 - 09/2004	236	59
SLD13	021650905	Reedy River	11/2004 - current	251	59
SLD18	02167000	Mainstem	10/1926 - current	1355	117
SLD19	02167450	Little River	03/1990 - current	224	30
SLD22	02167582	Bush River	02/1990 - current	114	26
SLD25	02168504	Mainstem	10/1988 - current	2418	169.5
SLD27	02169500	Mainstem	10/1939 - current	7849	178
SLD29	02169570	Gills Creek	10/1966 - current	59	7
SLD32	02169670	Cedar Creek	11/1980-09/1985	68	9

The reliability of past water supply to meet specific water user demands is an important consideration in the calibration process to ensure that water user demands and supply portfolios are properly represented in the model, as well as providing checks on supply availability at specific points of withdrawal. Timeseries plots, both monthly and daily, are used to assess the model's ability to simulate observed temporal variation and patterns in flow and storage data and to capture an appropriate range of high and low flow values. Percentile plots are useful for assessing the model's ability to reproduce the range of flows, including extreme events, observed in the past (and are particularly important when considering that the value of a long-term planning model like this is its ability to predict the frequency at which future flow thresholds might be exceeded, or the frequency that various amounts of water will be available). Monthly statistics provide valuable information on the model's ability to generally reproduce seasonal patterns, while annual totals and period of record mean flows help confirm the overall water balance represented in the model. Lastly, regulatory low flows (7Q10) are of specific interest as the model could be used to predict such low flows as a function of future impairment. However, the limitations of the daily model and supporting data should be properly considered in assessing model performance on this particular metric. Note that for the purposes of this exercise a simplified 7Q10 calculation was employed. Our approach used the Excel



percentile function to estimate the 10 year recurrence interval (10th percentile) of modeled and measured 7 day low flows. This differs from the more standard methods often using specific fitted probability distributions (e.g. log-Pearson).

Assessment of performance and adequacy of calibration was primarily based on graphical comparisons (modeled vs. measured) of the metrics described above. It is our opinion that graphical results, in combination with sound engineering judgement, provide the most comprehensive view of model performance for this type of model. Reliance on specific statistical metrics can result in a skewed and/or shortsighted assessments of model performance. In addition to the graphical assessments, period of record flow averages and 7Q10 values were assessed based on tabular comparisons and percent differences. Ultimately, keeping in mind the philosophies and objectives described in Section 7.1, consideration was given as to whether the model calibration could be significantly improved with further parameter adjustments, given the limited calibration "knobs" available in the process. In actuality, a clear point of "diminishing returns" was reached whereby no significant improvements in performance could be achieved without either: a) adjusting parameters outside of their range of uncertainty or, b) constructing an overly prescriptive historical model that then becomes less useful for future predictive simulations. At this point, the calibration exercise was considered completed.

7.3 Results

Detailed monthly and daily model calibration results are provided in **Appendix A** and **B**, respectively. In general, a strong agreement between modeled and measured data is observed for all targeted sites. Discrepancies between modeled and measured flow data are generally within the reported range of uncertainty associated with the USGS flow data used to drive the models (5 – 20%) (USGS http://wdr.water.usgs.gov/current/documentation.html). Seasonal and annual patterns in both flow and reservoir storage data are reproduced well by the model. Monthly fluctuations (timeseries) and extreme conditions (percentiles) are also very well reproduced by the model for most sites.

Not surprisingly, the poorest fit occurs at the flow gage directly below Lake Murray (SLD25). This was expected as the flow at this site is governed almost entirely by lake operations and management decisions, rather than natural hydrology. Lake operations are represented in the model by a simplified set of operating rules that are assumed to be consistently followed and do not factor in human decision-making. Consequently, reproducing monthly or daily flows at this location was expected to be more challenging than at other sites. That being said, an excellent agreement in average flow (+1%) and monthly percentiles (within $\pm \sim 20\%$) is achieved by the model, confirming that long-term statistics are well captured. Additionally, the general pattern of high and low flow periods is very well represented by the model.

For all sites, modeled mean flow values, averaged over the full period of record, were all within 1% of measured mean flows. This indicates that the overall water balance is very well simulated in the model and there are no obvious missing or excess sources of flow in the model. Reservoir storage simulations, while clearly simplified, appear to be accurately replicating historical ranges and patterns of reported storage, particularly for the two largest reservoirs in the basin (Lake Greenwood and Lake Murray). Exceptions to this, as noted above, appear to be largely attributable to anomalies in reservoir operations likely associated with reservoir construction or maintenance activities.

Monthly flow percentiles are also well captured by the model across nearly all sites. Monthly flow percentile deviations are all generally within 10 - 20% with no clear bias one way or the other.



In terms of daily timestep simulations, daily flow fluctuations are generally well captured by the model – in some cases surprisingly well (see SLD09 and SLD 27), given the lack of reach routing. The exception, again, is SLD25 (below Lake Murray), for reasons described above. These challenges are undoubtedly amplified for the daily timestep model. Modeled daily percentile plots exhibit excellent agreement with measured data for upstream mainstem locations (SLD04 and SLD09) and all tributary locations. For SLD18 (Chappells), the model generally slightly under predicts daily flows for the 80th through the 95th percentile and then over predicts daily flows for the highest percentiles (> 95th). These discrepancies are likely primarily attributable to the lack of reach routing and overall simplified representation of hydrologic processes in the model, common to all water allocation models. However, these discrepancies are within 20% of gaged flows and deemed acceptable for the daily model.

Modeled regulatory low flow values (7Q10) are within 2% of measured values at mainstem gages SLD04, SLD18 and SLD25. For SLD09 and SLD27, the model over predicts the 7Q10 by approximately 35%, which is deemed acceptable for this challenging metric, especially because the volume of water associated with the SLD09 deviation is very small and the available record of annual low flows is limited at SLD27 (for reasons described above). Further, it is important to realize that low flows in the model are highly sensitive to modeled basin water use and operations. Small errors in estimated (or reported) withdrawals or modeled reservoir releases can have a significant impact on modeled annual low flows. Consequently, model uncertainty associated with this metric is relatively high and additional model adjustments to improve this calibration fit are not justified.

Lastly, the model adequately hindcasts delivered water supply for each of the water users in the model. Simulated supply roughly equals simulated demand for all users, with no significant shortfalls. An exception to this is Duke Power's Lee Steam Station (which was retired in 2014) where there was significant uncertainty in reported and hindcasted withdrawal data due to the complex nature of their water use and lack of high-quality records (Ed Bruce, Duke Power, pers. comm., Aug 2015). Therefore, this historical shortfall was not rectified in the model. Additionally, some of the minor water users in the basins, primarily agricultural and golf course irrigators, show periodic shortfalls in the model during particularly low flow periods. For these instances, it is likely that reported or assumed surface water usage is inaccurate and irrigation was temporarily reduced due to supply limitations.



Section 8

Use Guidelines for the Baseline Model

The baseline Saluda River Basin Model will be located on a cloud-based server which can be accessed using a virtual desktop approach. Interested stakeholders will be provided access to the model by DNR and/or DHEC upon completion of a model training course. Current plans are for training to be offered to stakeholders once the models for all eight river basins are completed.

This model will be useful for the following types of scenarios:

- Comparison of water availability resulting from managed flow (future or current) to unimpaired flow throughout the basin.
- Comparison of current use patterns to fully permitted use of the allocated water (or any potential future demand level), and resulting flow throughout the river network.
- Evaluation of new withdrawal and discharge permits, and associated minimum streamflow requirements.
- Alternative management strategies for basin planning activities.

Users will also be able to change the duration of a model run in order to focus on specific years or hydrologic conditions. For example, the default model will run on a daily or monthly time step from 1925 through 2013 in order to test scenarios over the full historic period of recorded hydrologic conditions. In some cases, though, it may be useful to compile output over just the period corresponding to the drought of record, or an unusually wet period.

Flow conditions can also be changed by the user, though it will be important for the user to understand implications when unimpaired flows (naturalized flows) are replaced with other time series. In certain basins outside the Saluda, it will be useful to examine flows with either managed or unimpaired flows coming across state lines into South Carolina. In the Saluda Basin, it may be useful (for example) to alter boundary condition flows to test the impacts of potential climate variability.

Regardless of the type of scenario to be run, it is important to understand how to interpret the output. Whether running long-duration or short-duration runs, the output of the model will represent time series of flows, reservoir levels, and water uses. As such, the results can be interpreted by how frequently flow or reservoir levels are above or below certain thresholds, or how often demands are satisfied. This frequency, when extrapolated into future use, can then be translated into probabilities of occurrence in the future. It will be the user's responsibility to manipulate the output to present appropriate interpretations for the questions being asked, as illustrated in the following example:

Example: For a 10-year model run over a dry historic decade, a user is interested in knowing the frequency that a reservoir drops below a certain pool elevation. Results indicate that under current demand patterns, the reservoir will drop below this threshold in one month out of the ten years. Under future demand projections (modified by the user), the results indicate that the reservoir will drop below this threshold in six months during the driest of the ten years. If the results are presented annually, both



scenarios would be the same: a 10% probability of dropping below that level in any given year. If they are presented monthly, they will, of course, be different. Depending on the nature of the question, it will be important for users to be aware of how output can be used, interpreted, and misinterpreted.

Further guidance on use of the Model is provided in the *Simplified Water Allocation Model (SWAM) User's Manual* (CDM Smith, 2015). The User's Guide provides a description of the model objects, inputs, and outputs and provides guidelines for their use. A technical documentation section is included which provides detailed descriptions of the fundamental equations and algorithms used in SWAM.



Section 9

References

CDM Smith, 2015. Simplified Water Allocation Model (SWAM) User's Manual, Version 2.0.

Bruce, Ed (Duke Energy), August 2015. Personal Communication.

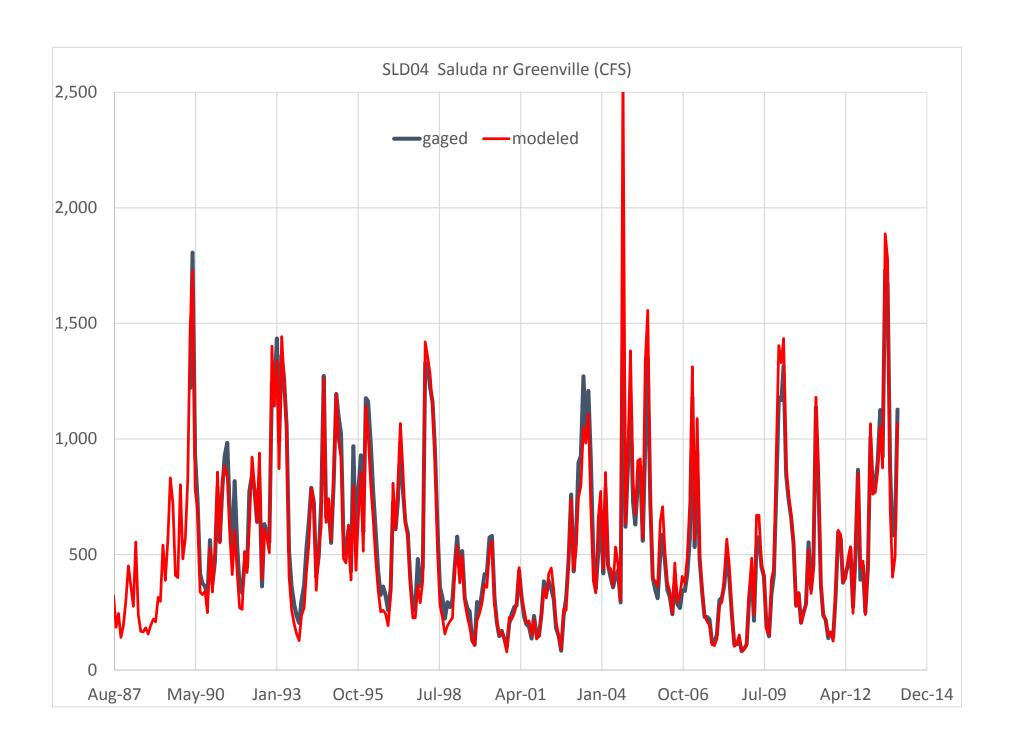
Torcellini, P., N. Long and R. Judkoff, 2003. *Consumptive Water Use for U.S. Power Production – Technical Report*. National Renewable Energy Laboratory (NREL/TP-550-33905).

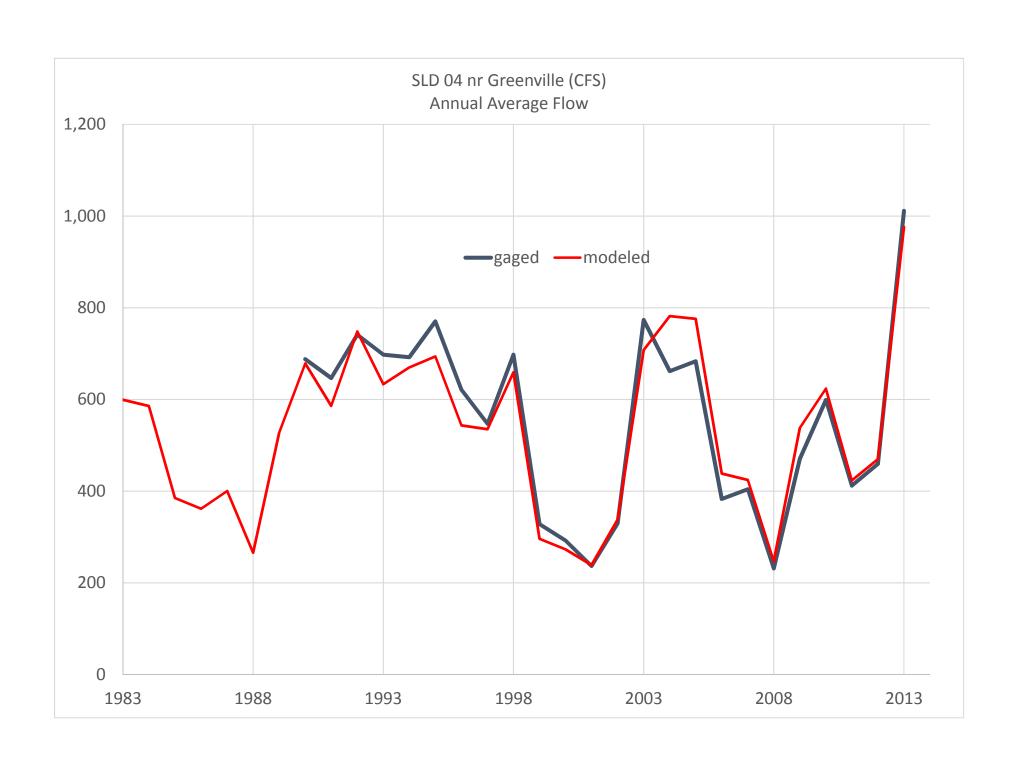


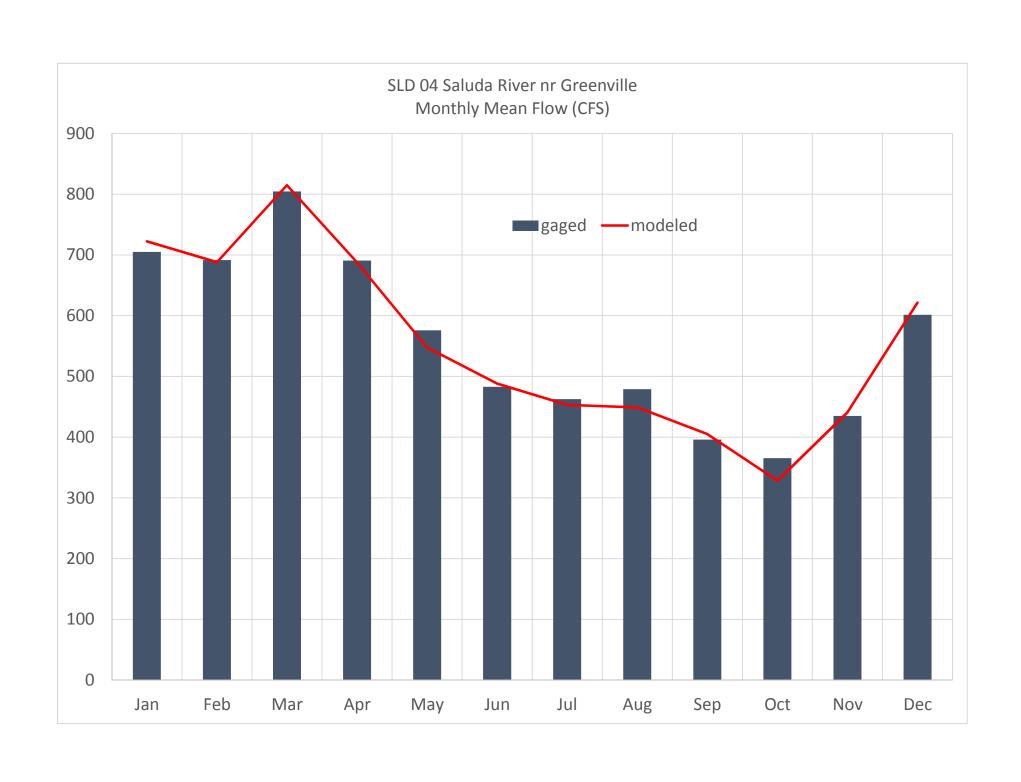
Appendix A

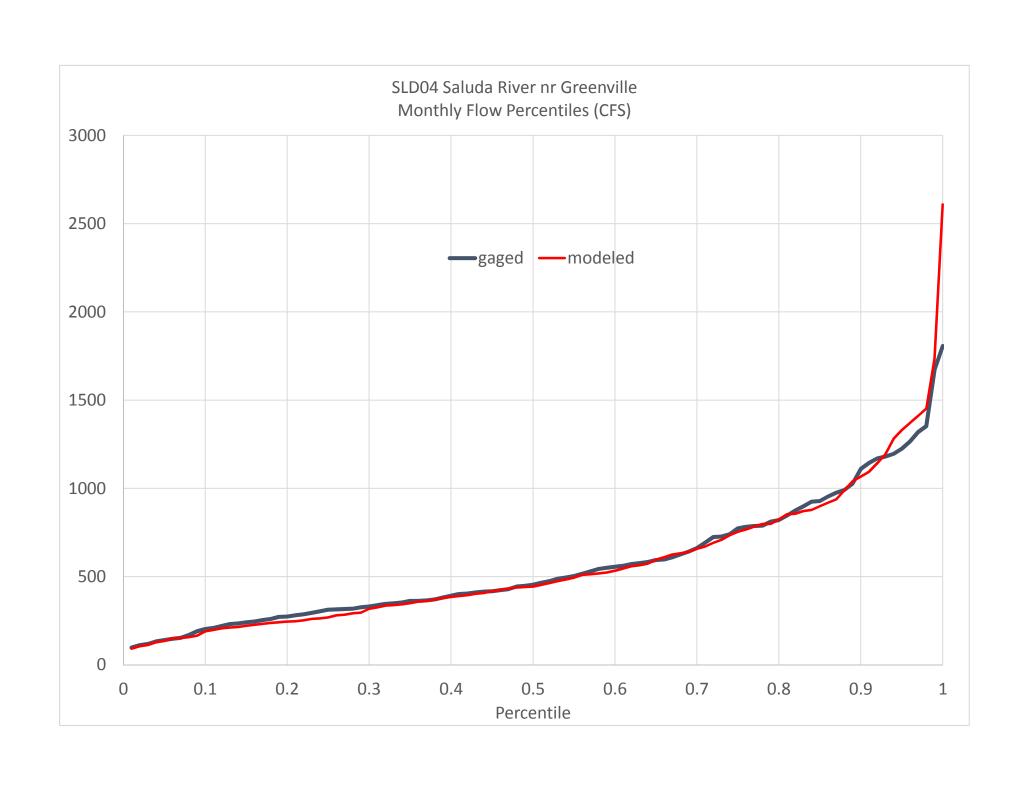
Saluda River Basin Model Monthly Calibration Results

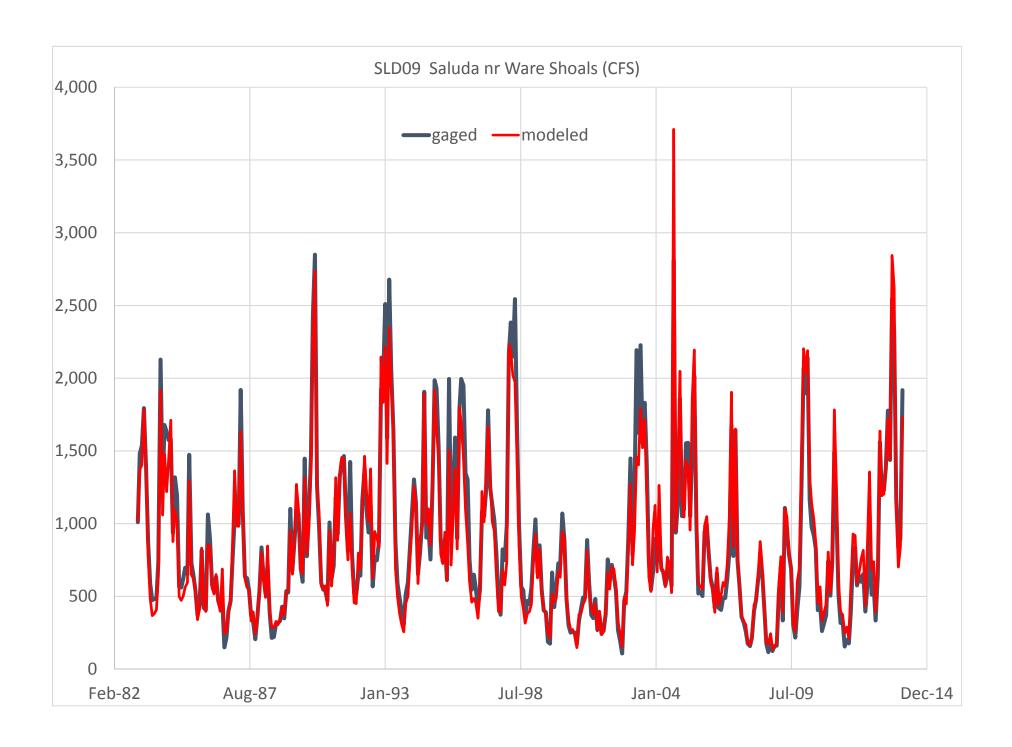


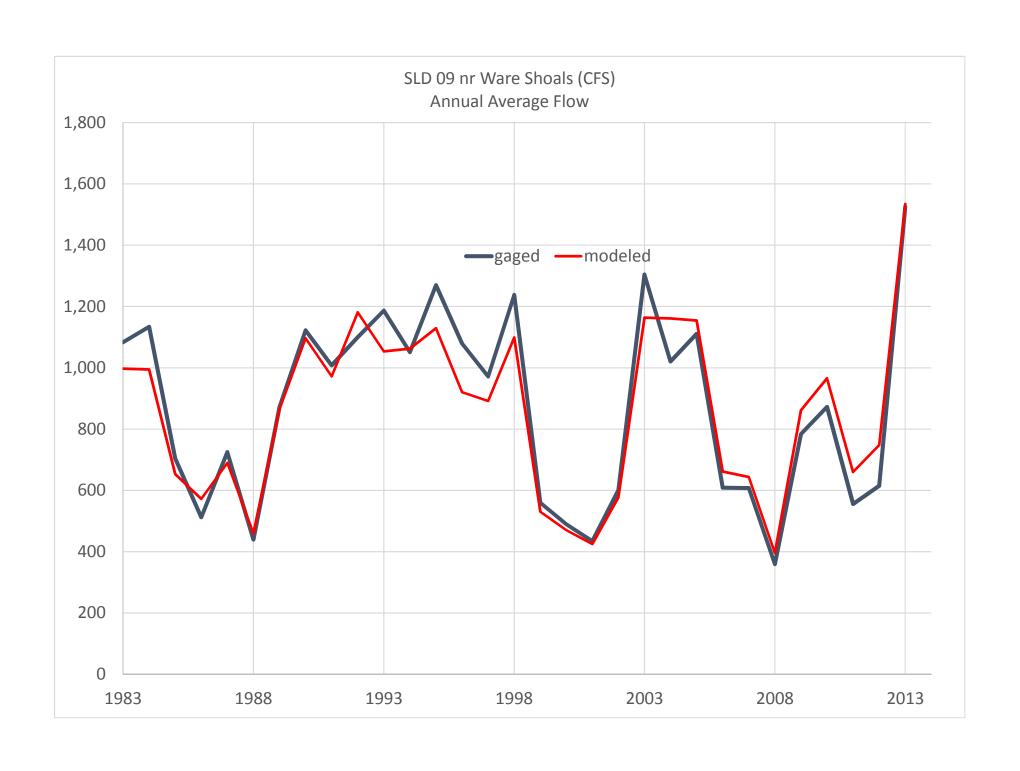


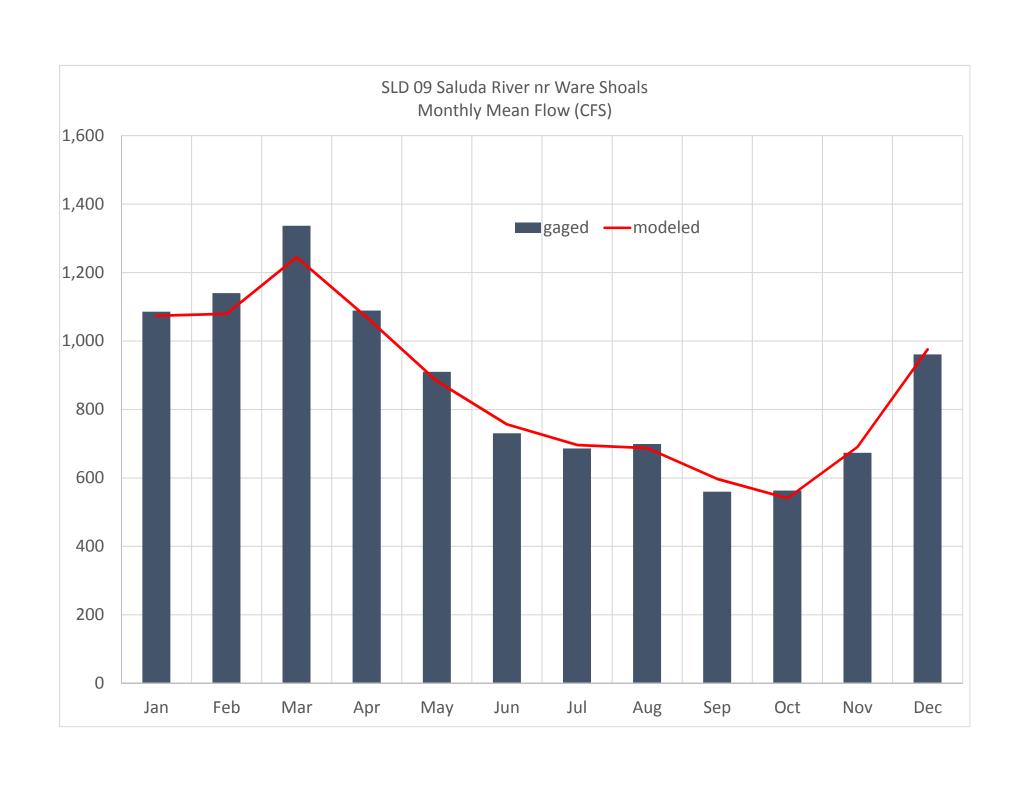


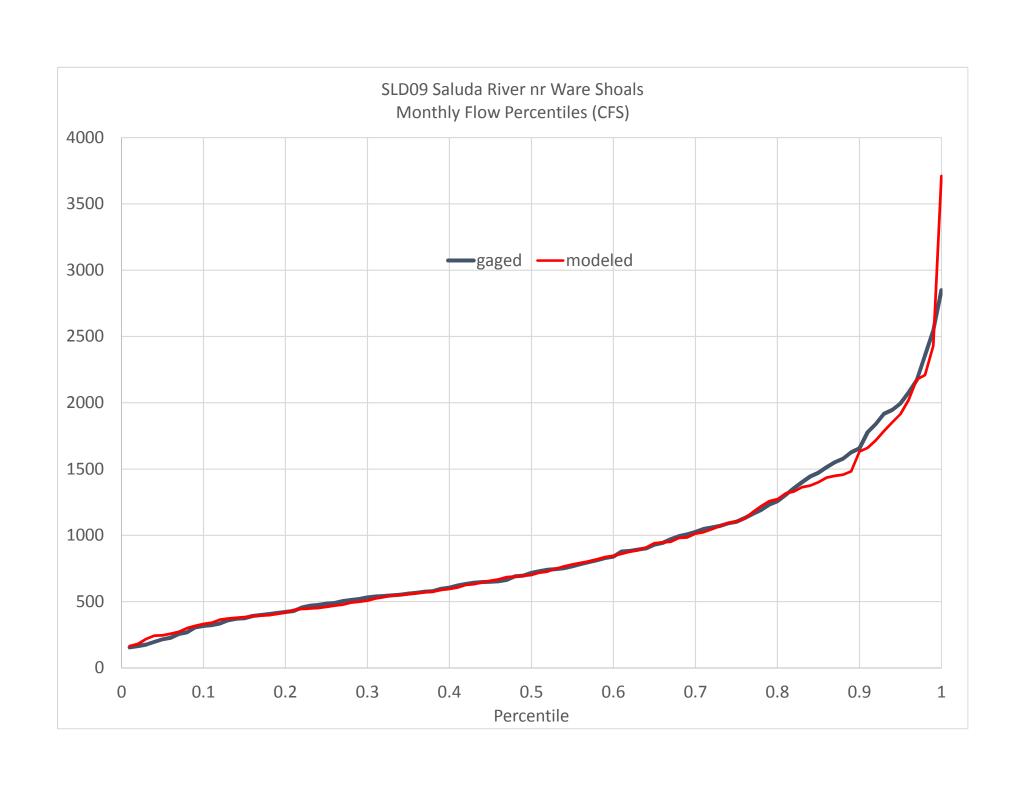


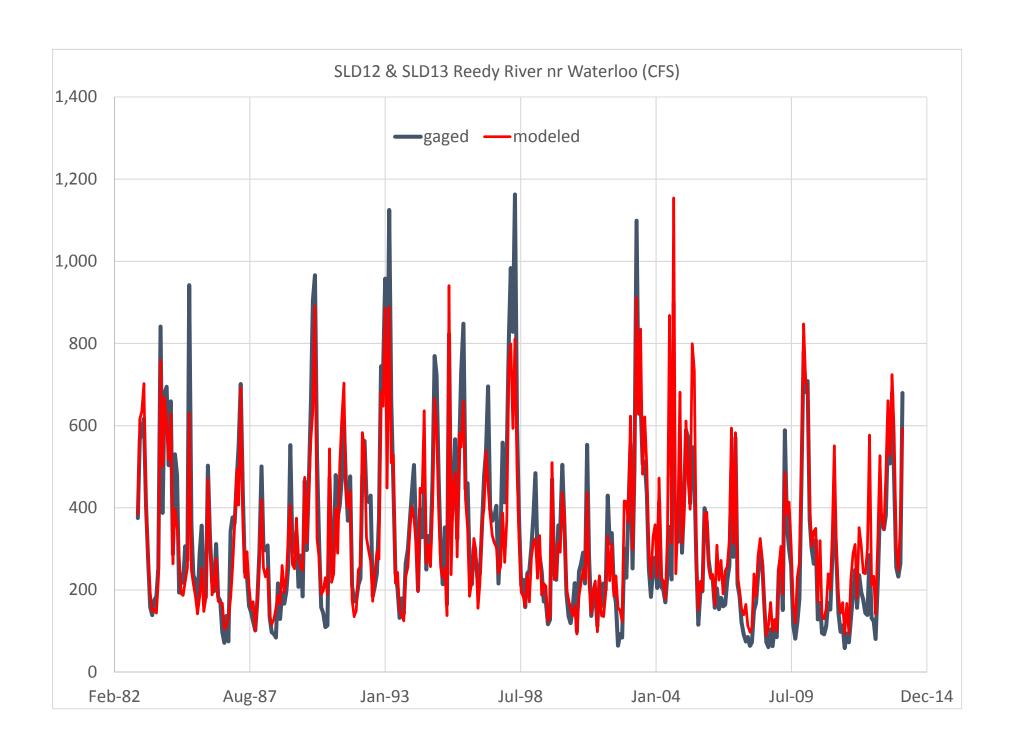


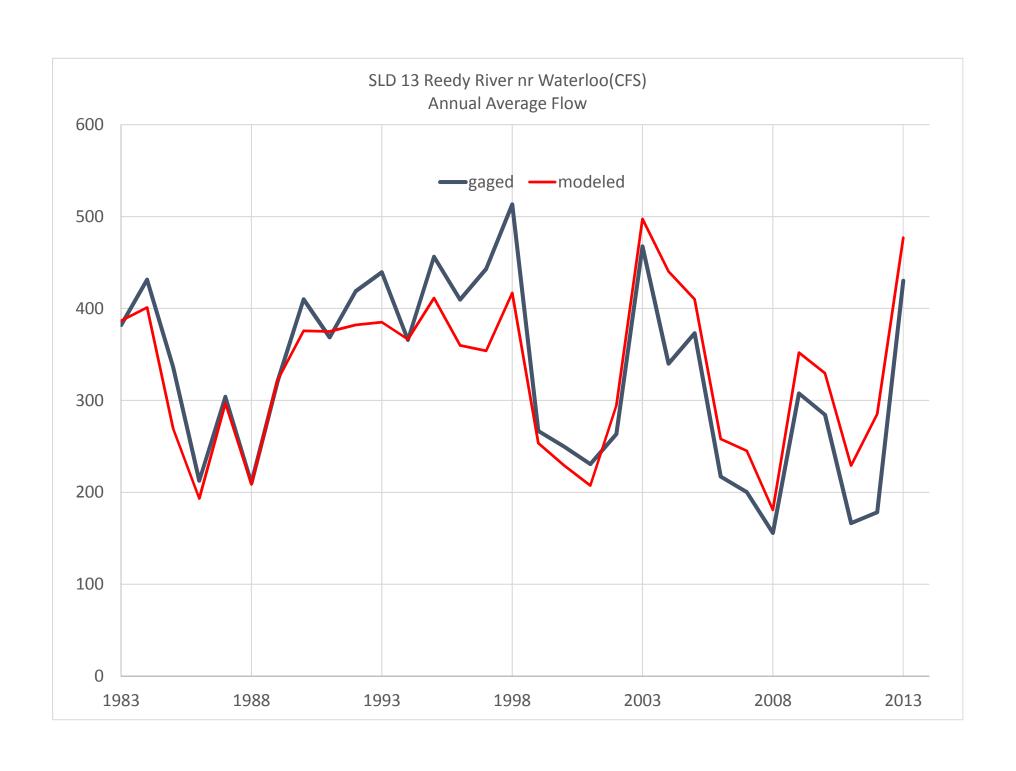


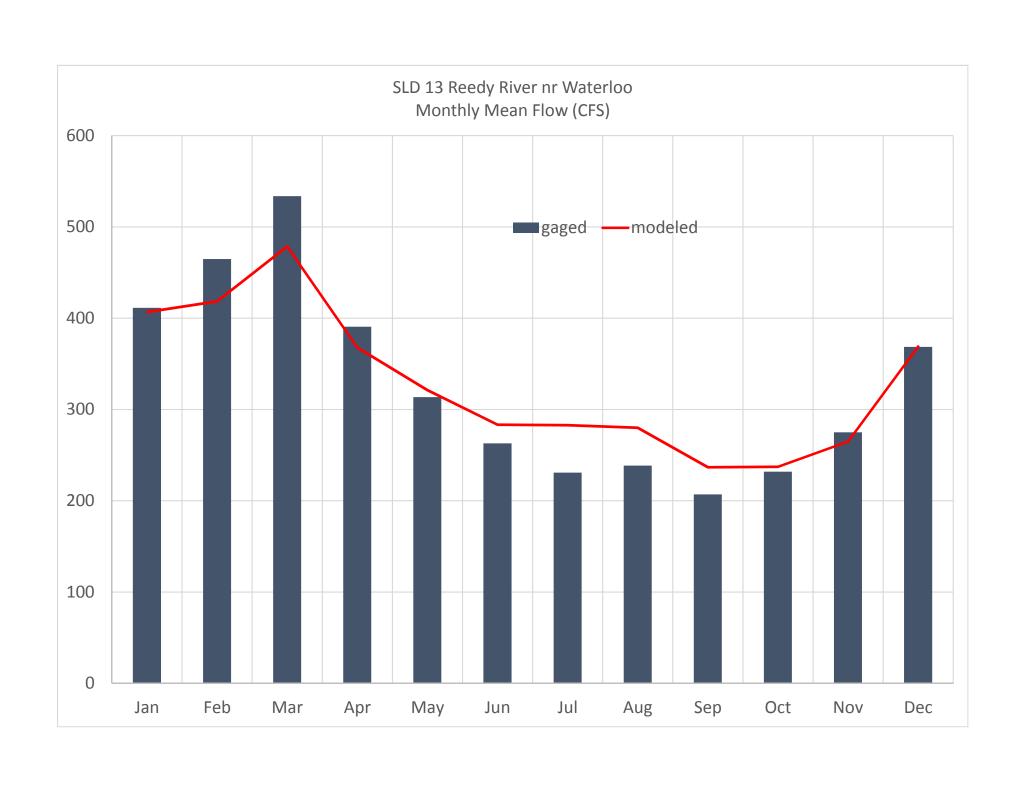


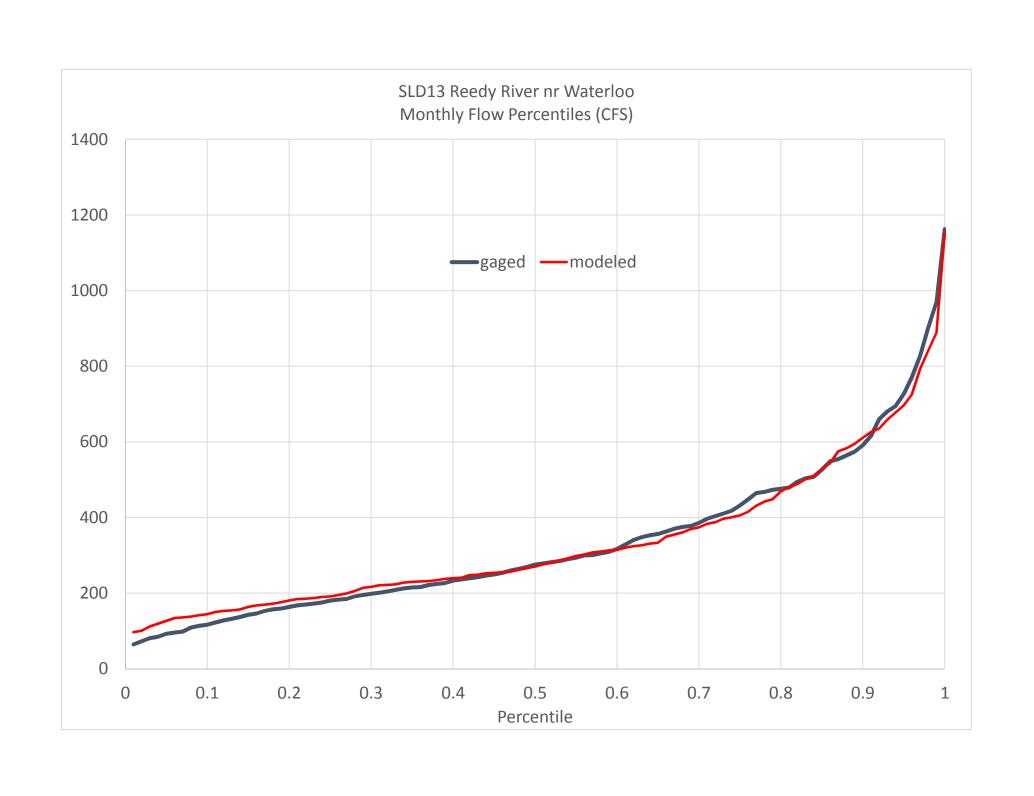


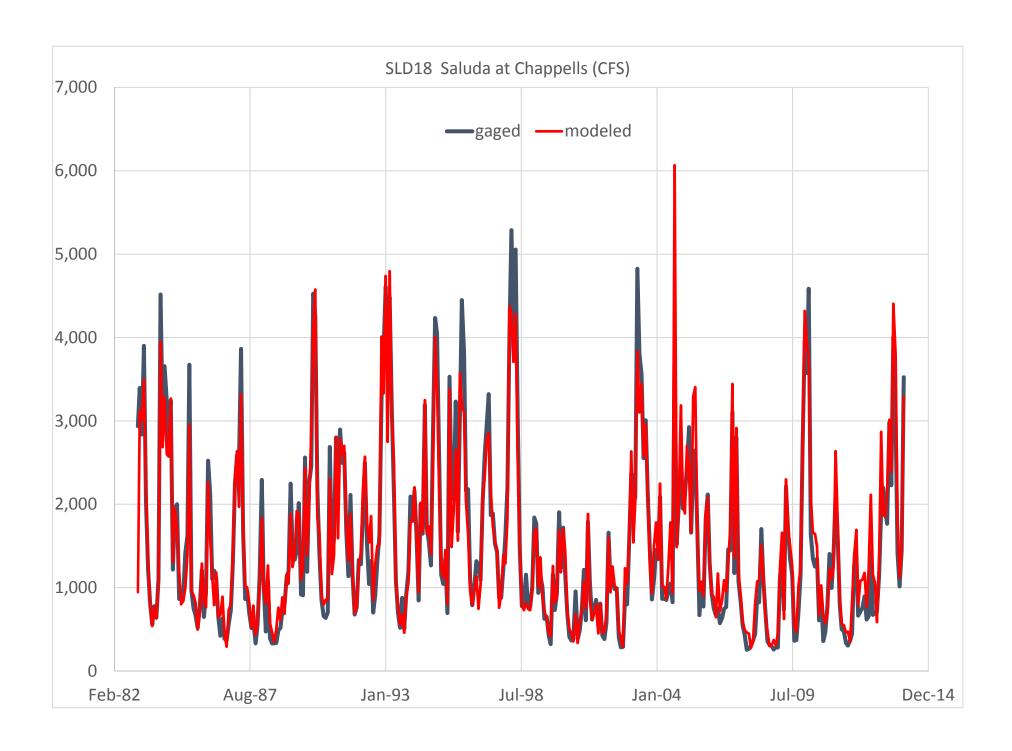


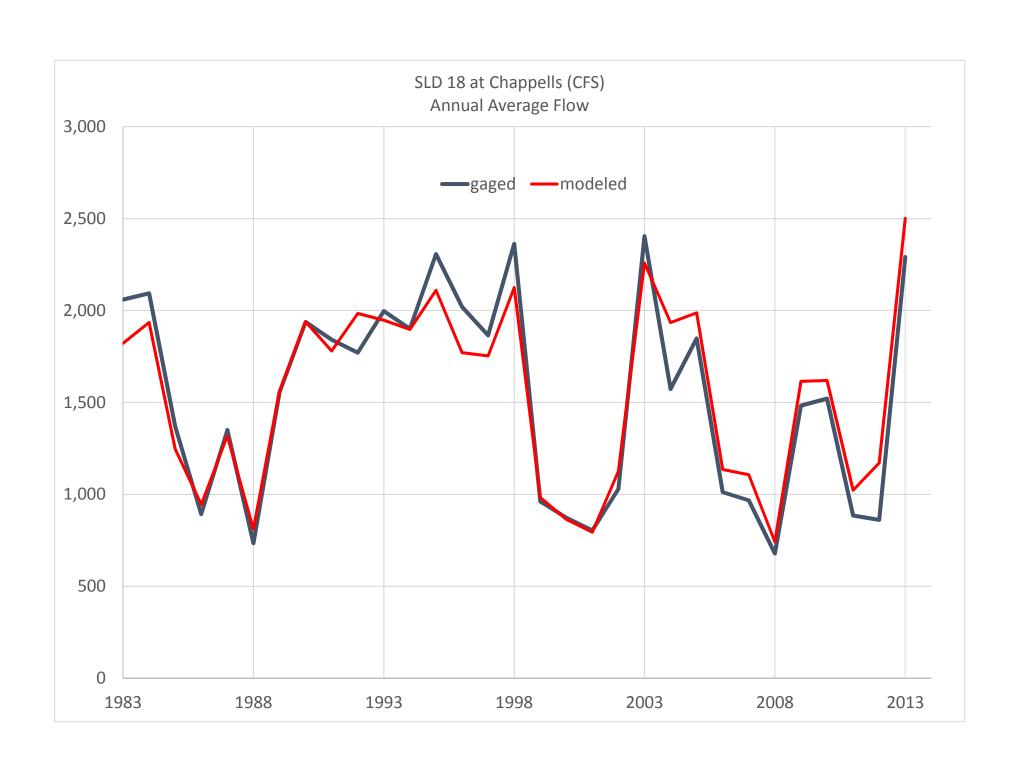


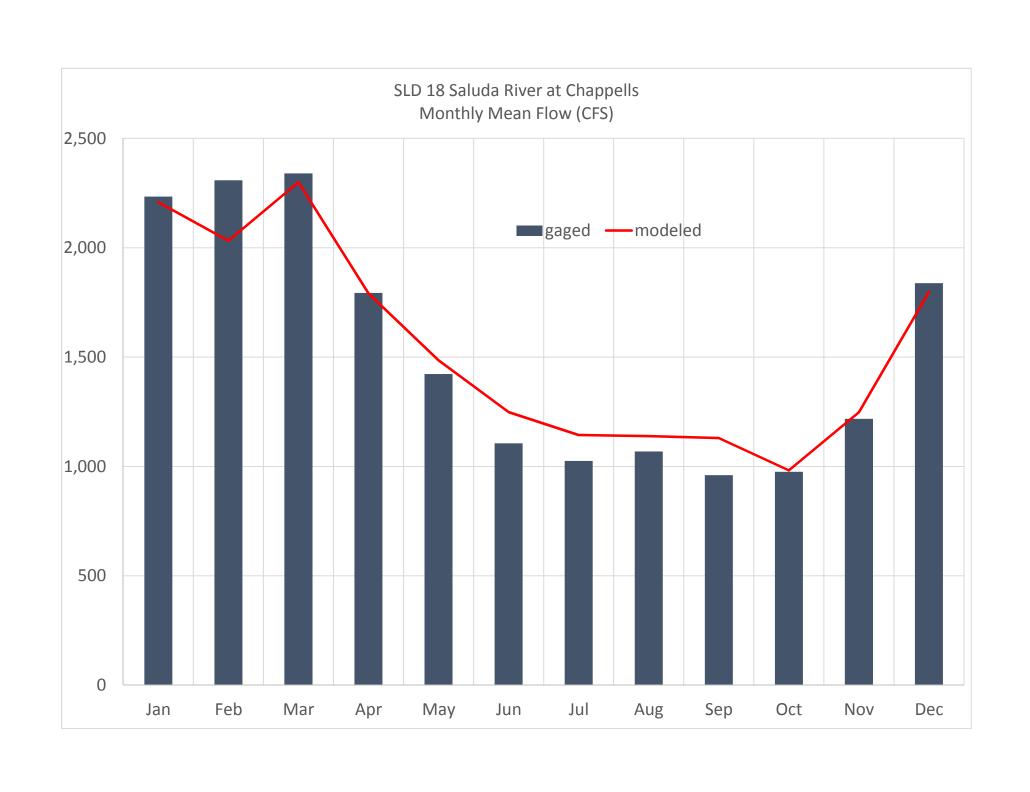


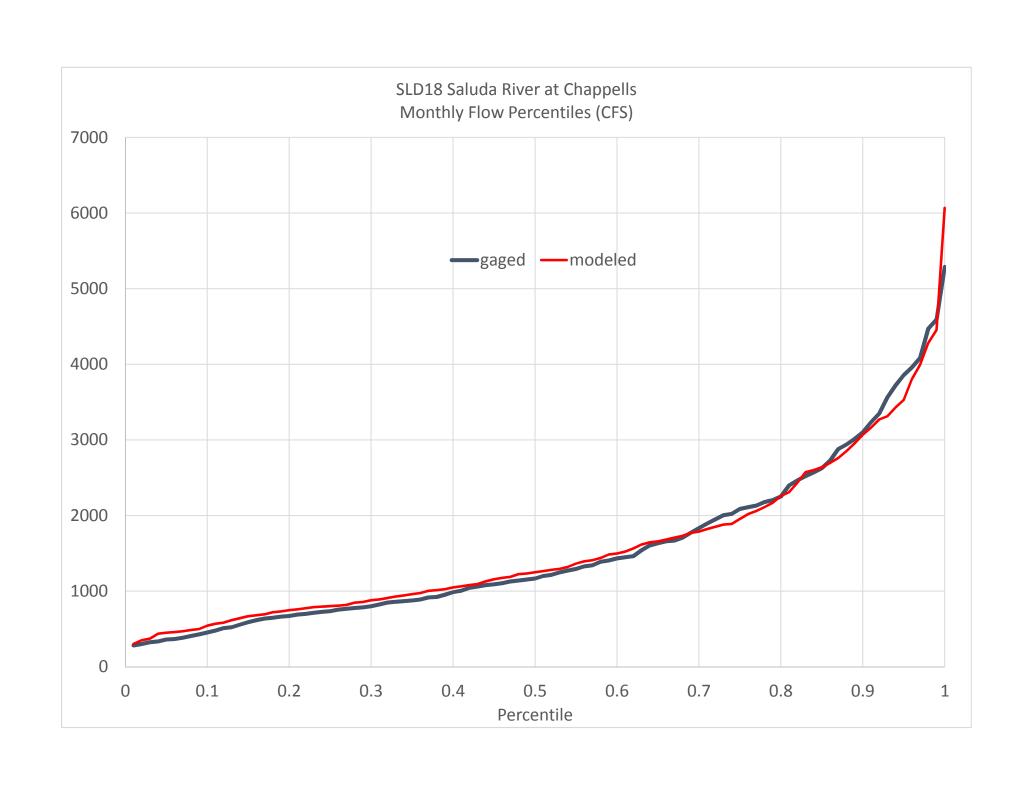


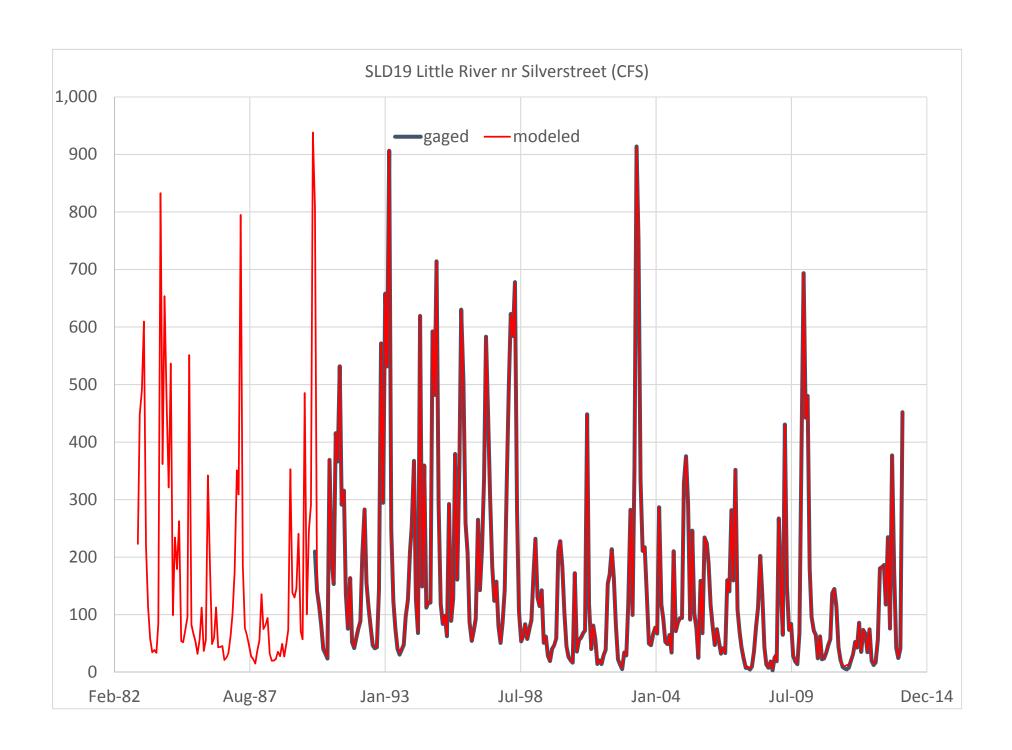


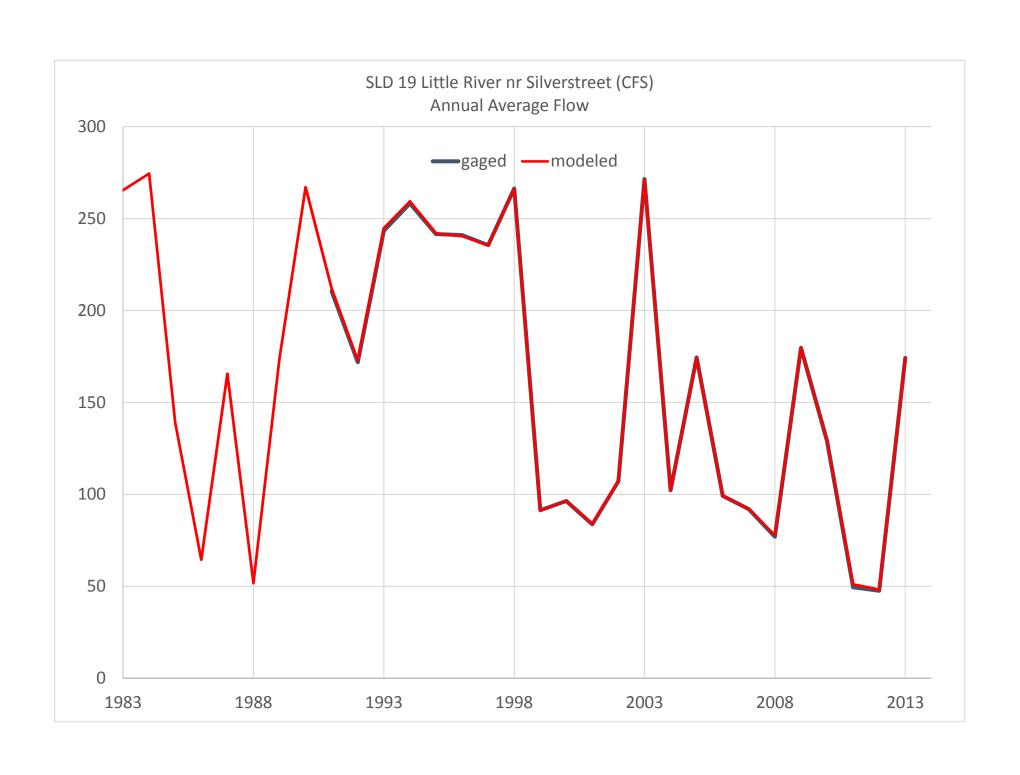


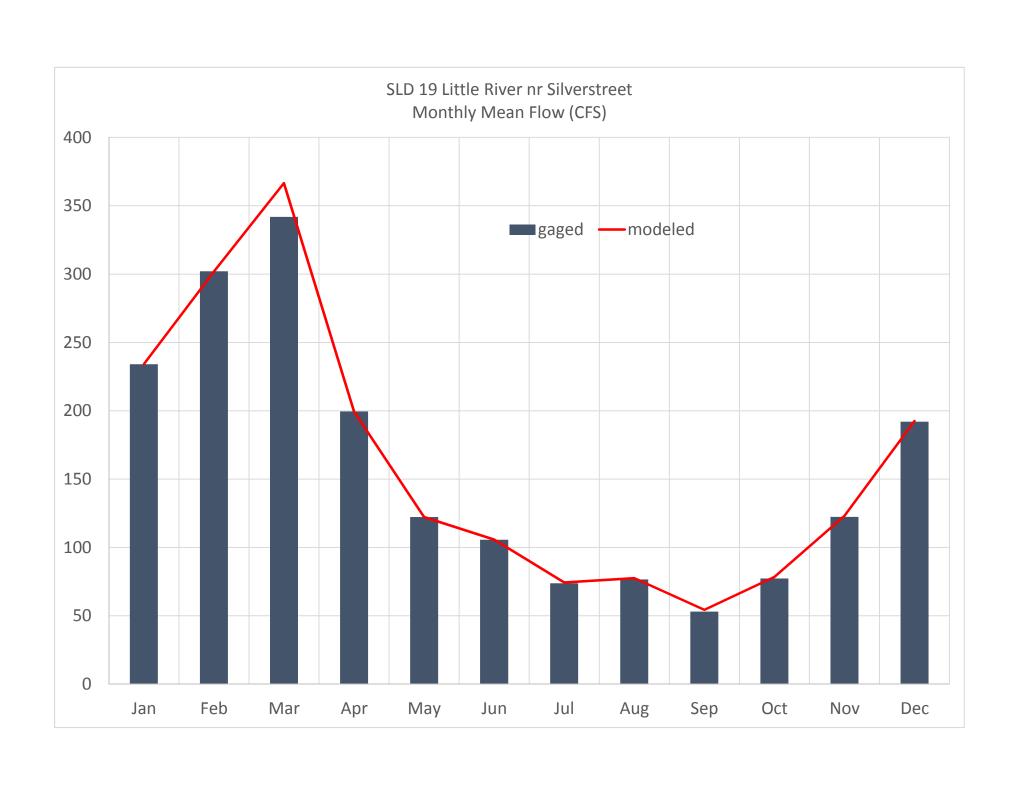


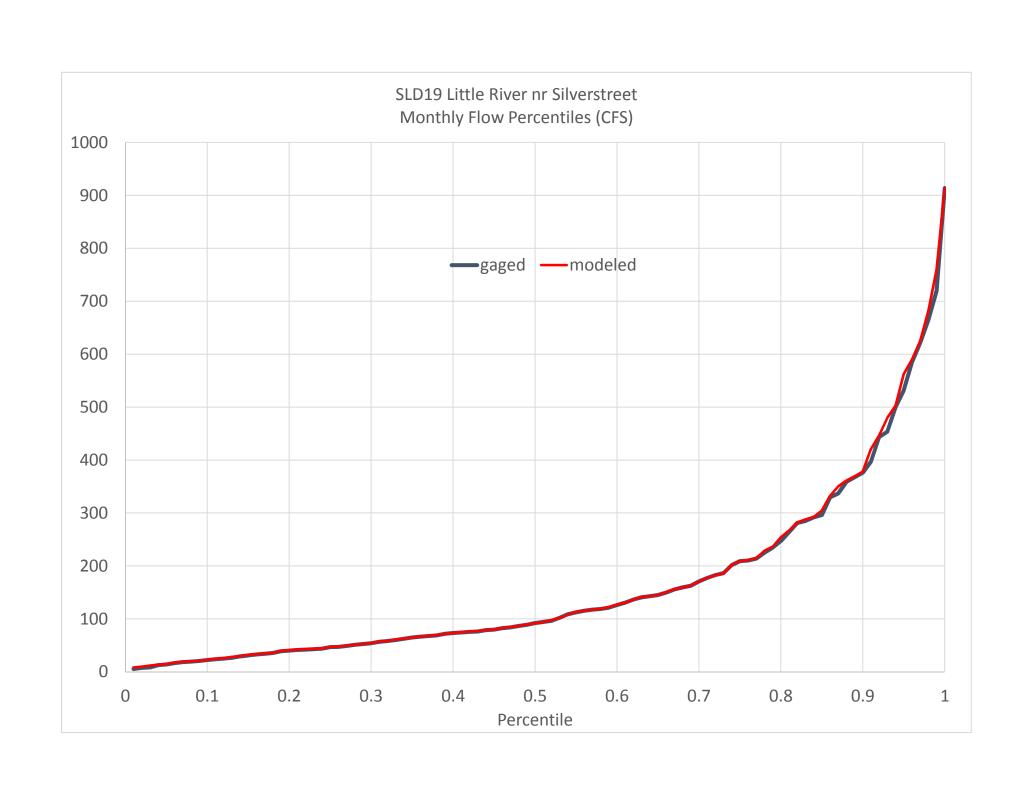


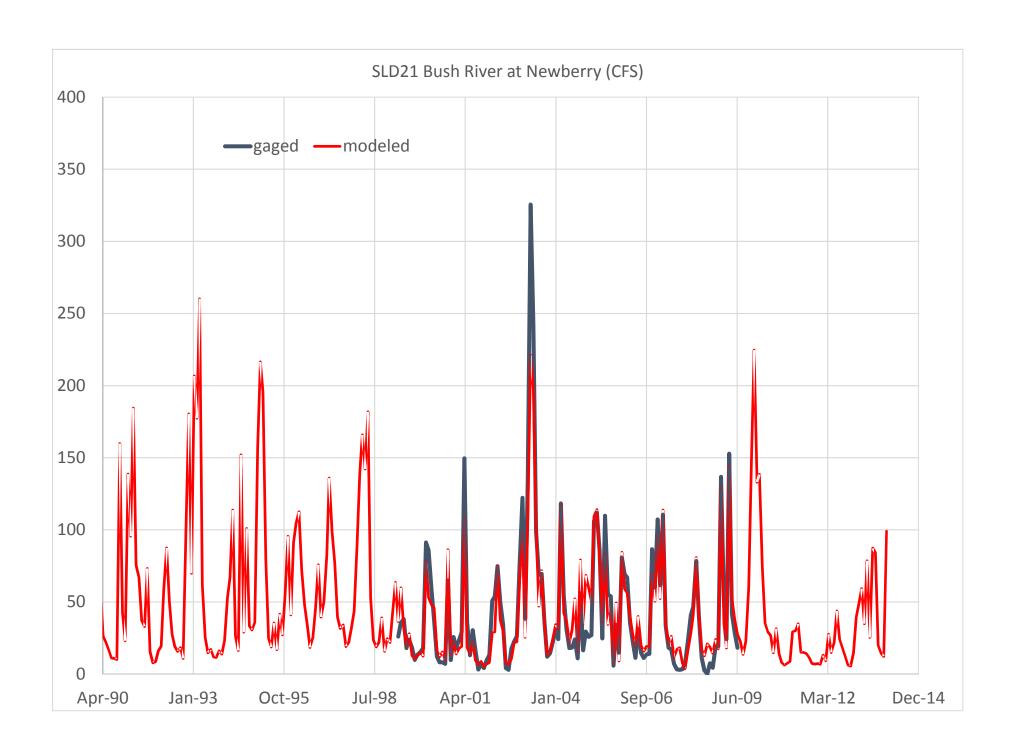


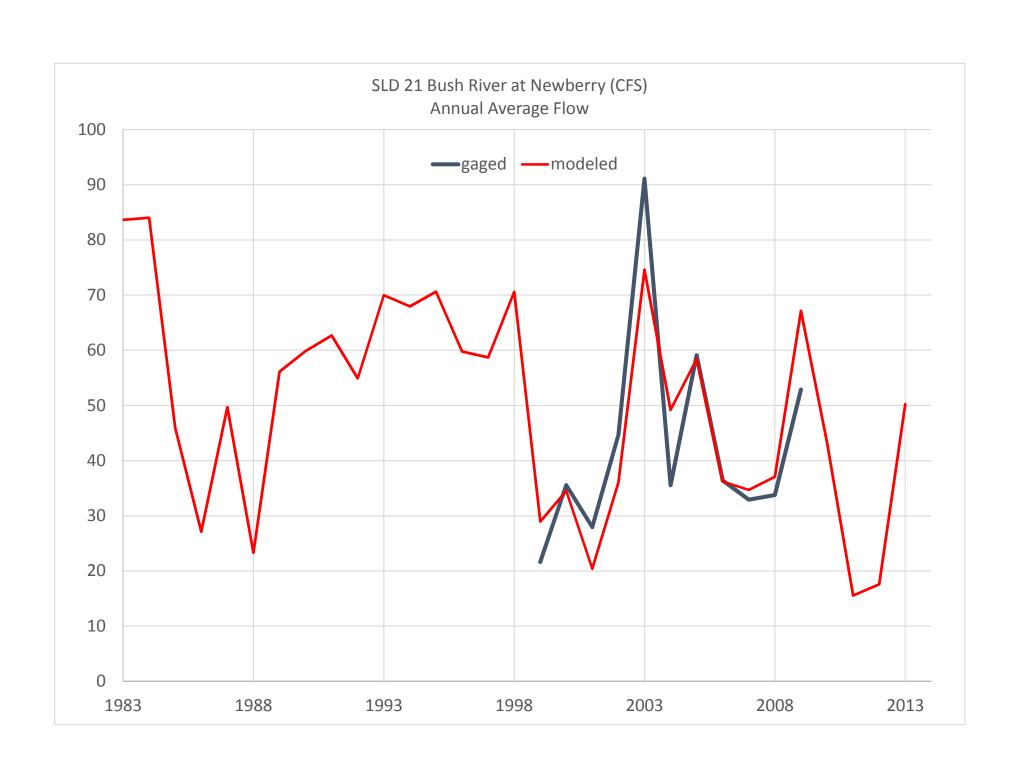


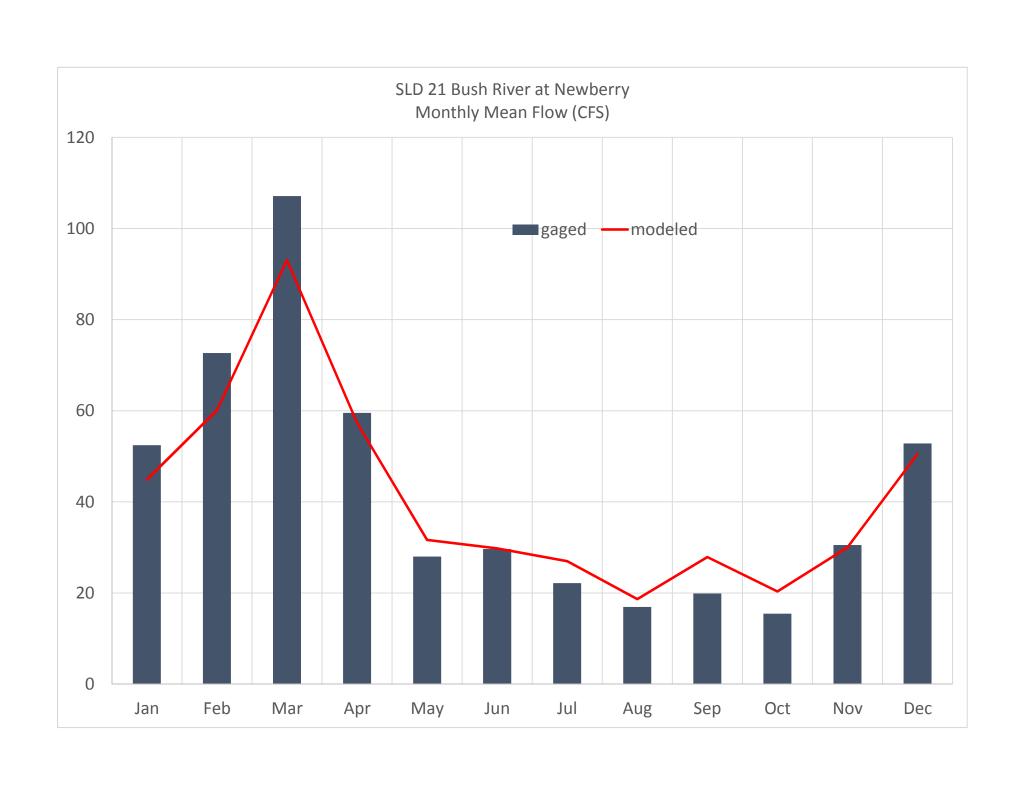


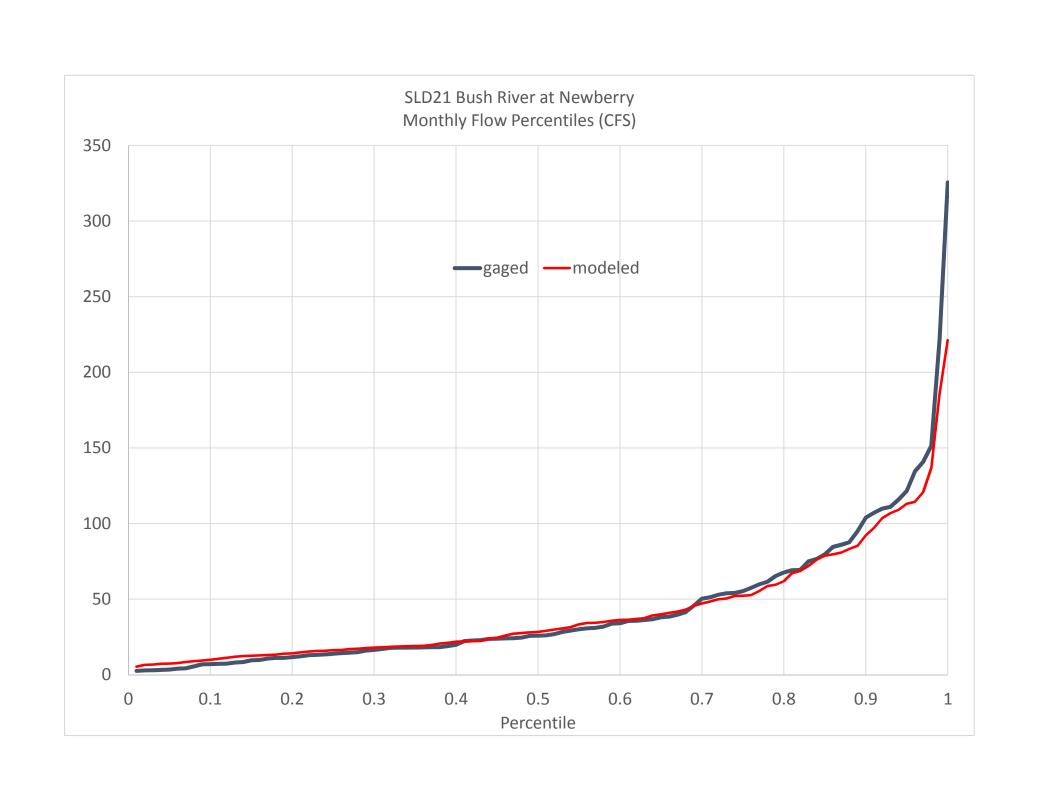


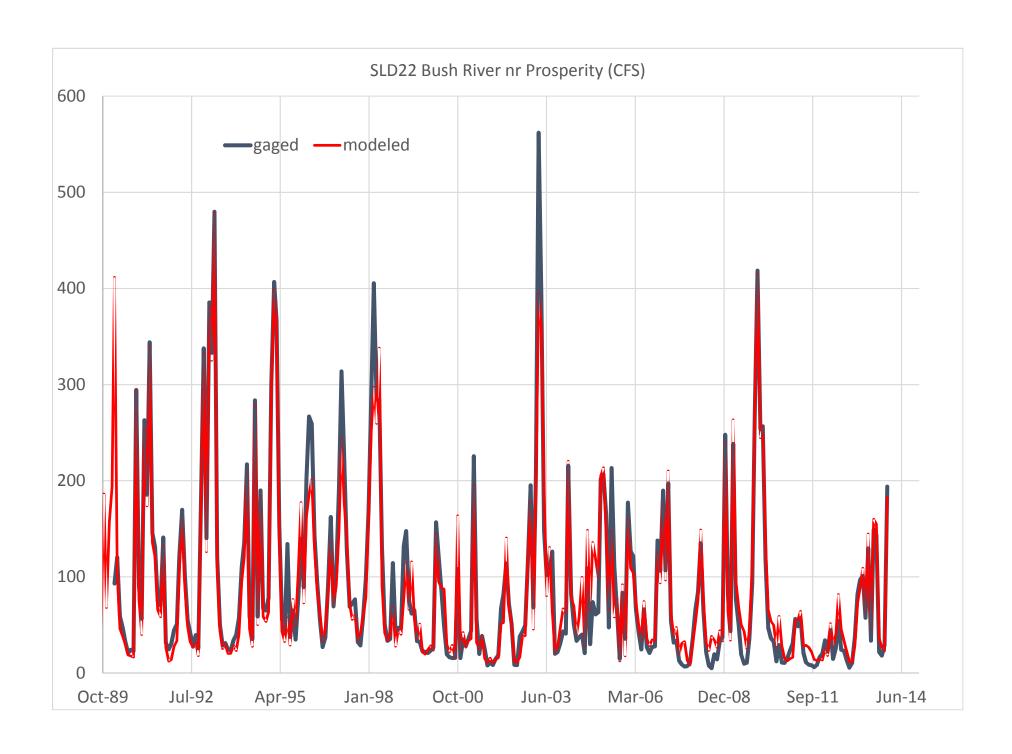


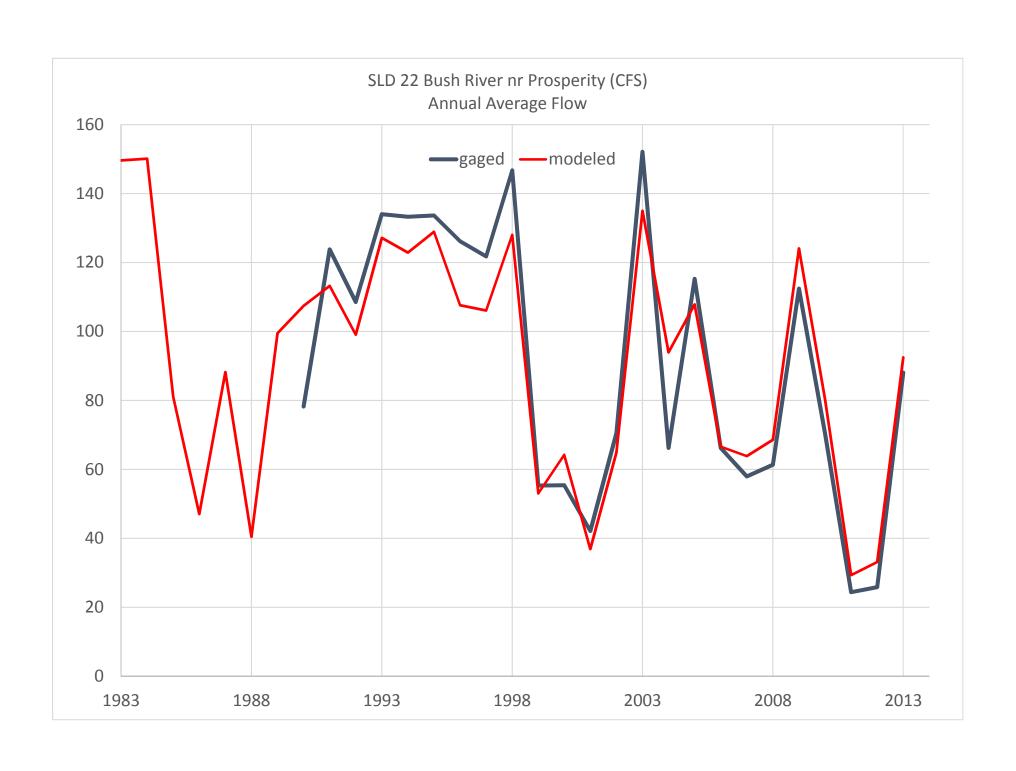


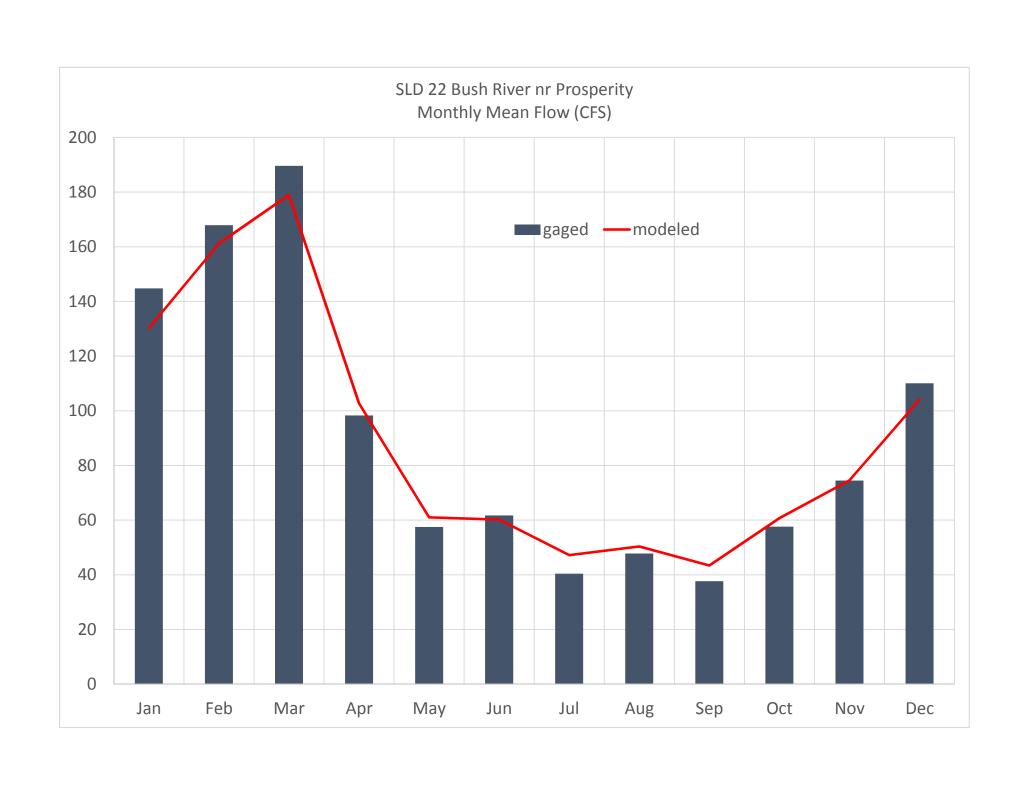


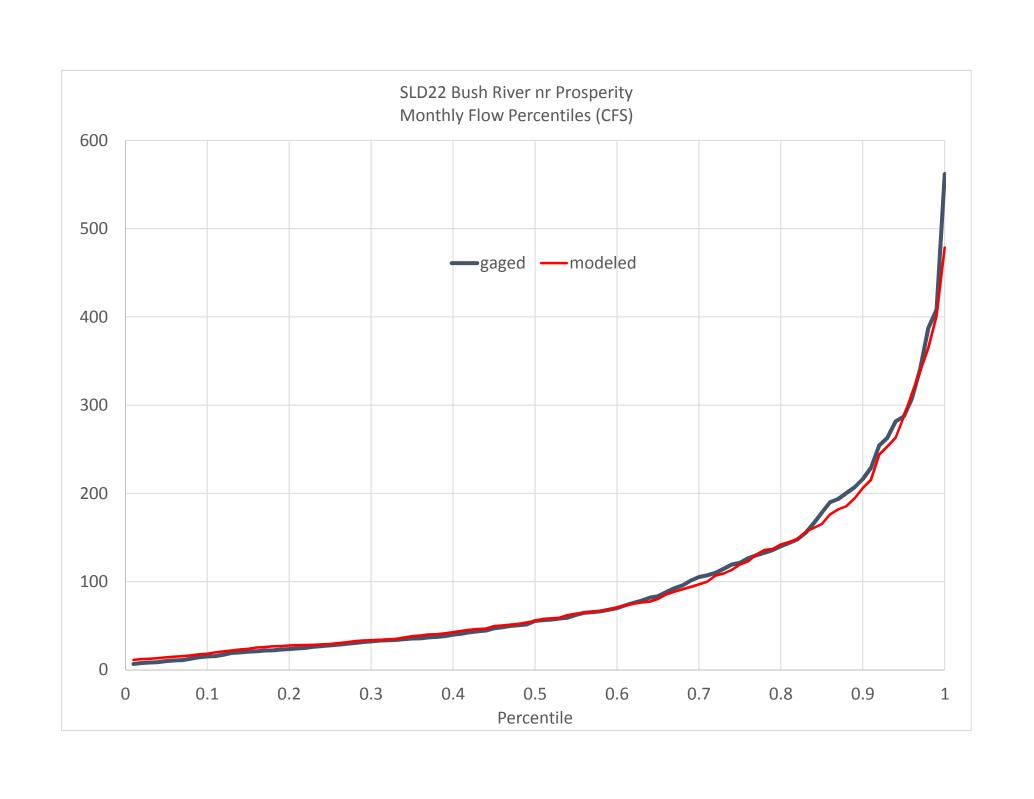


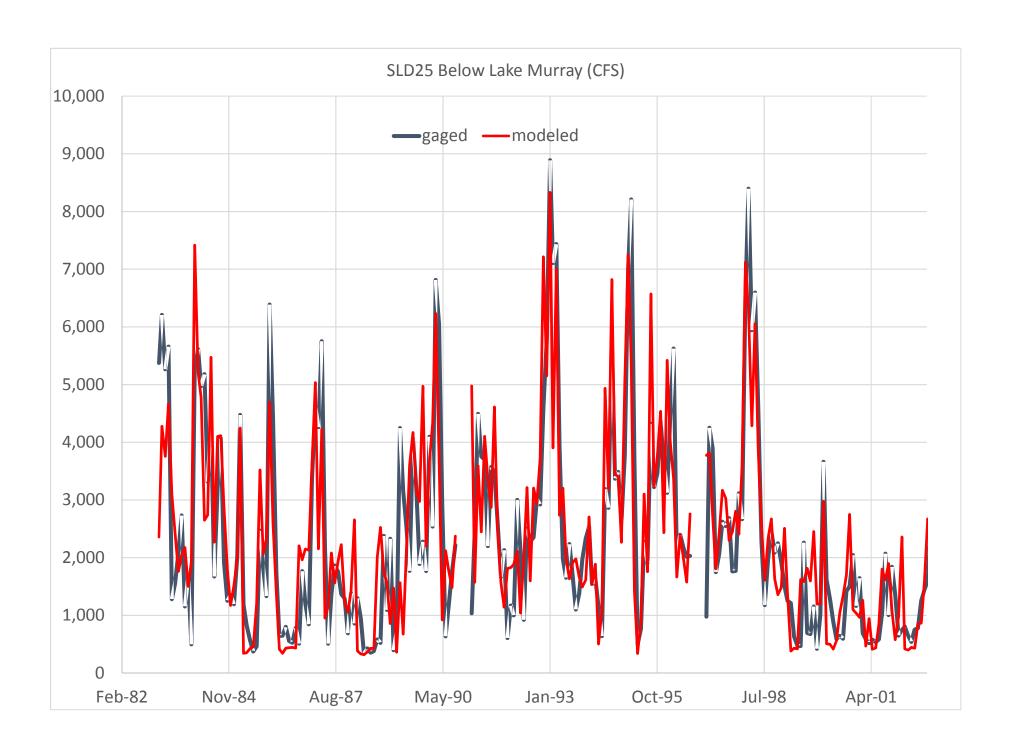


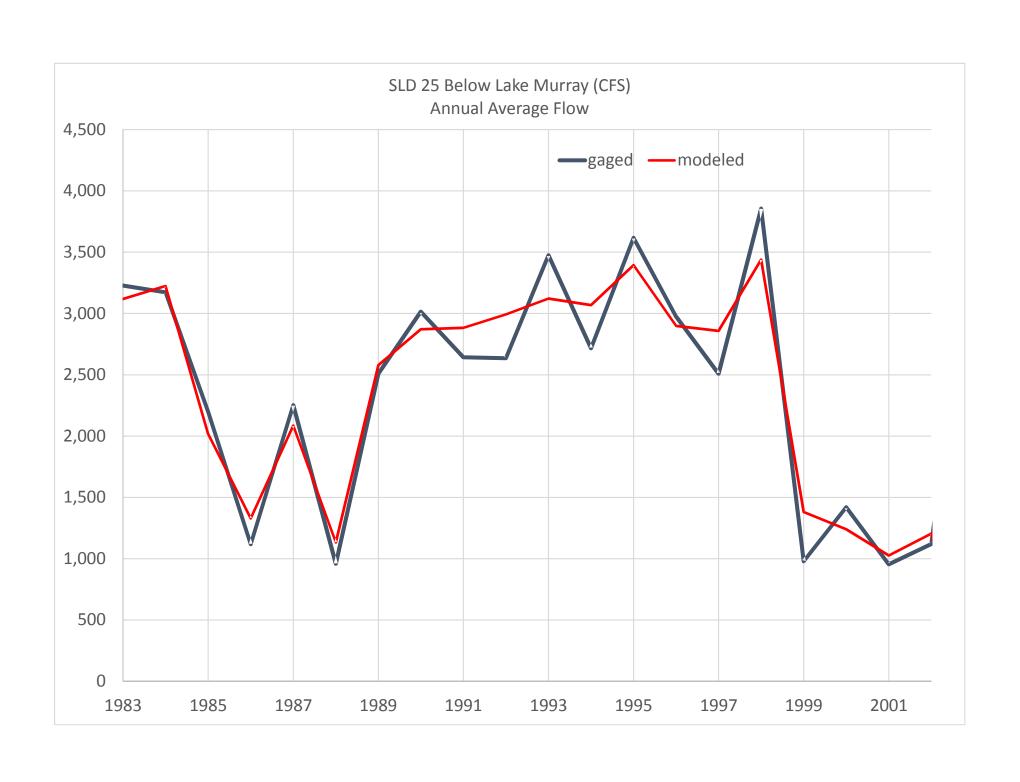


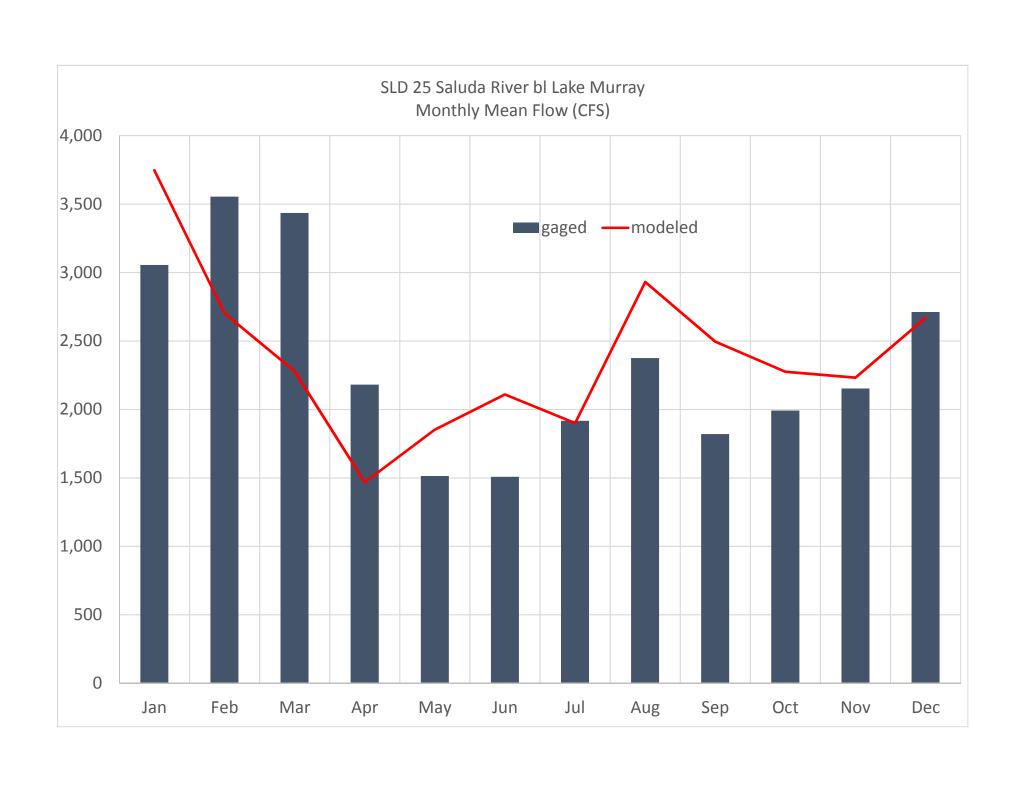


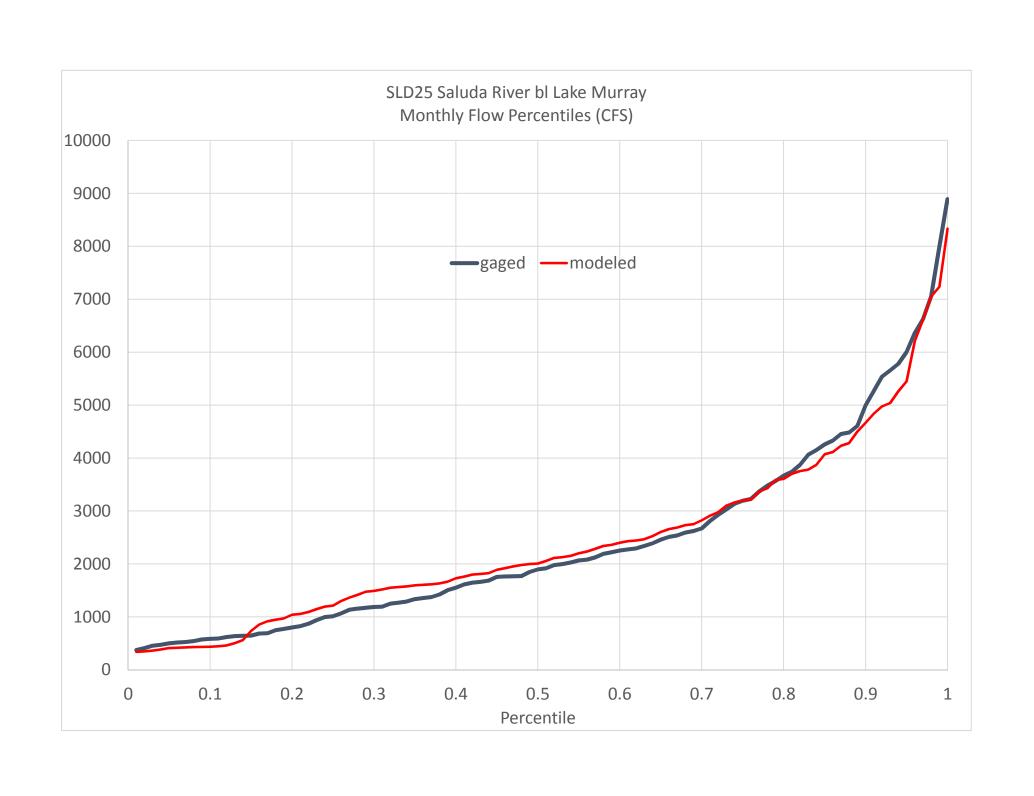


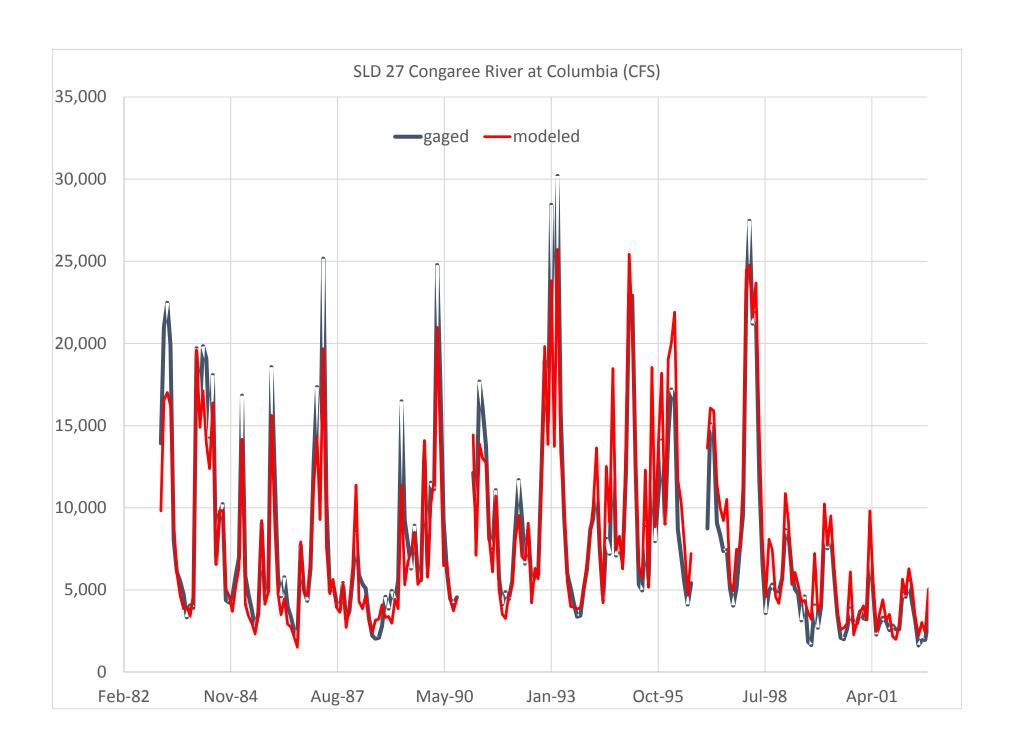


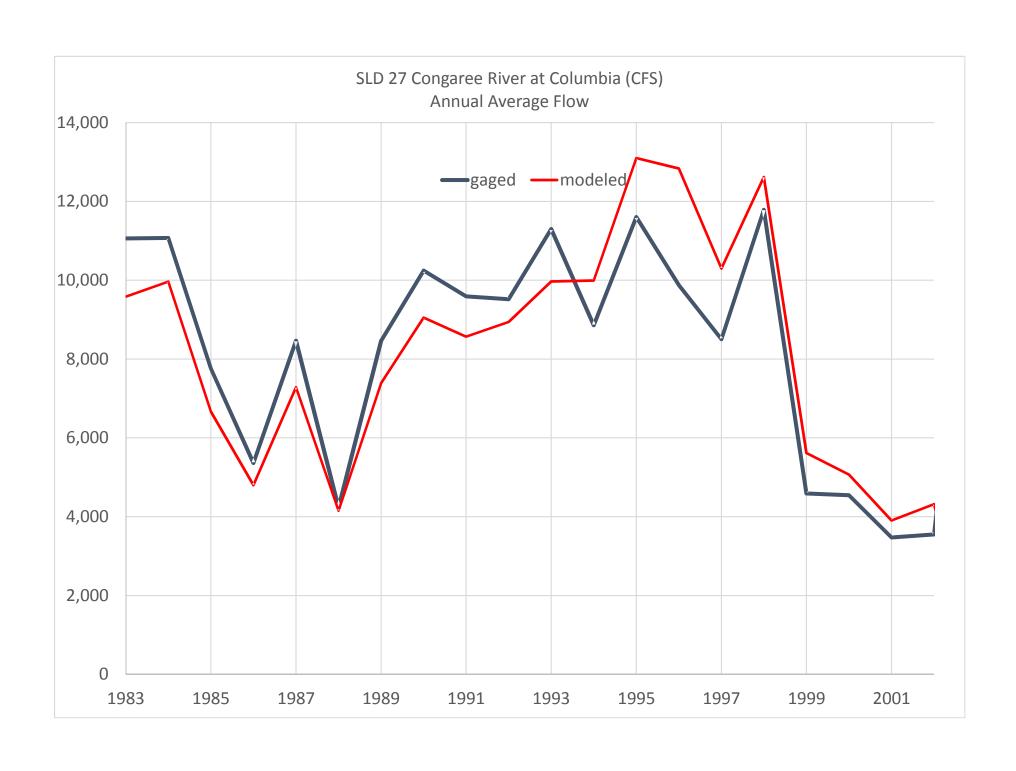


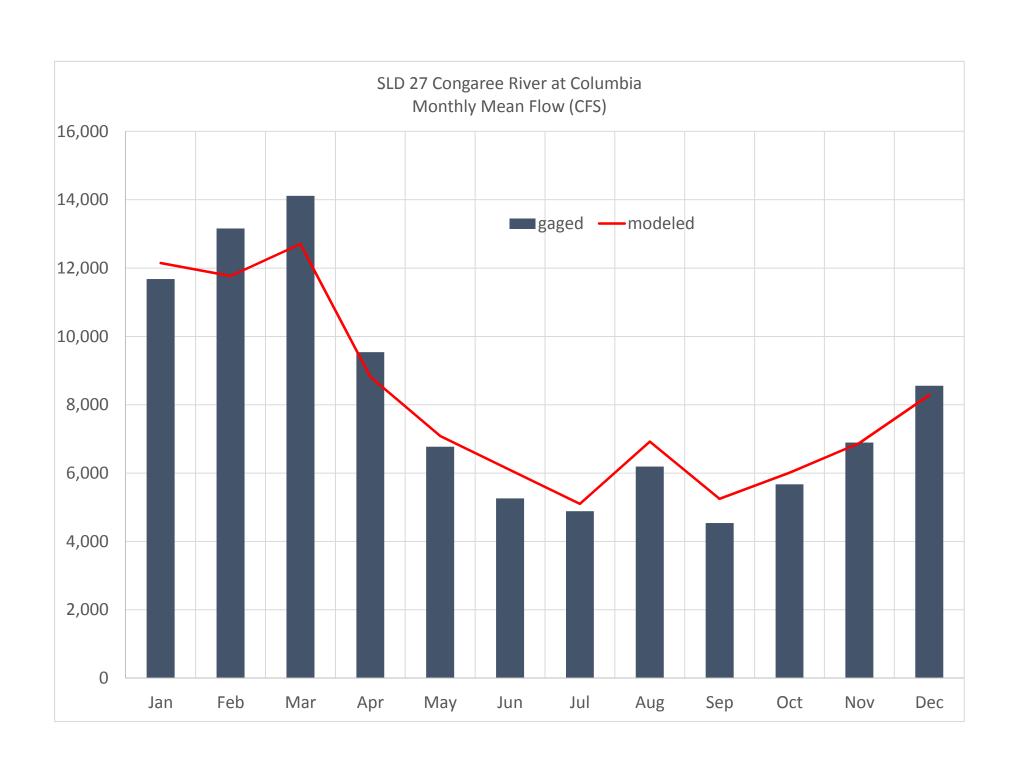


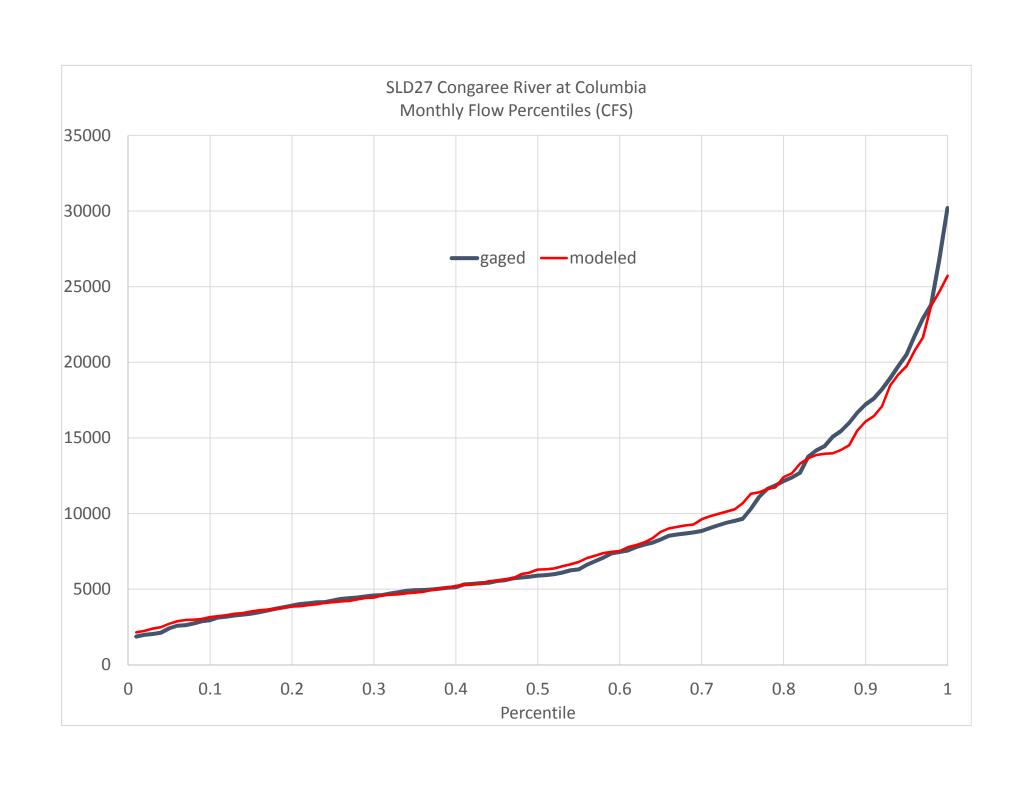


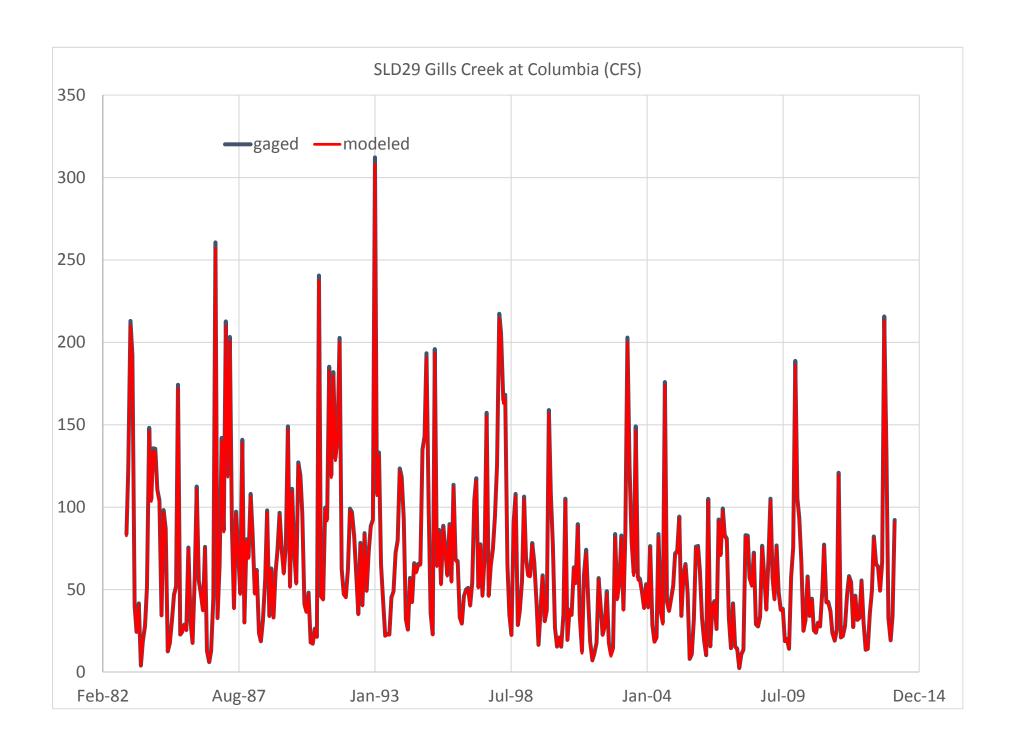


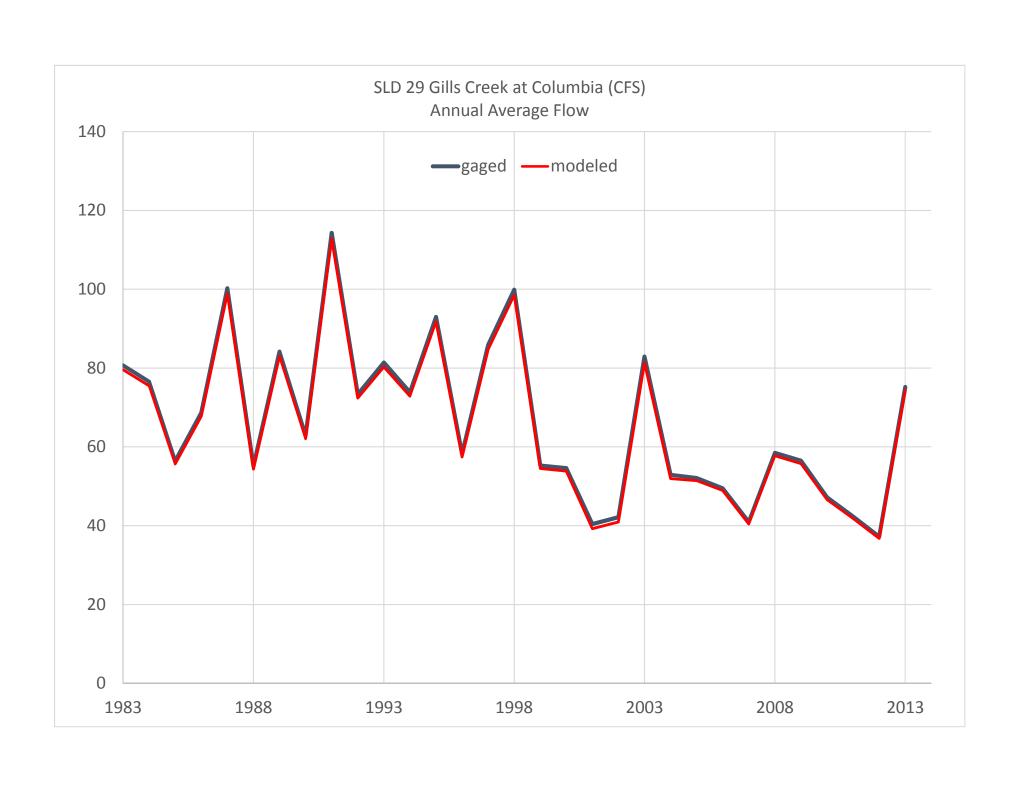


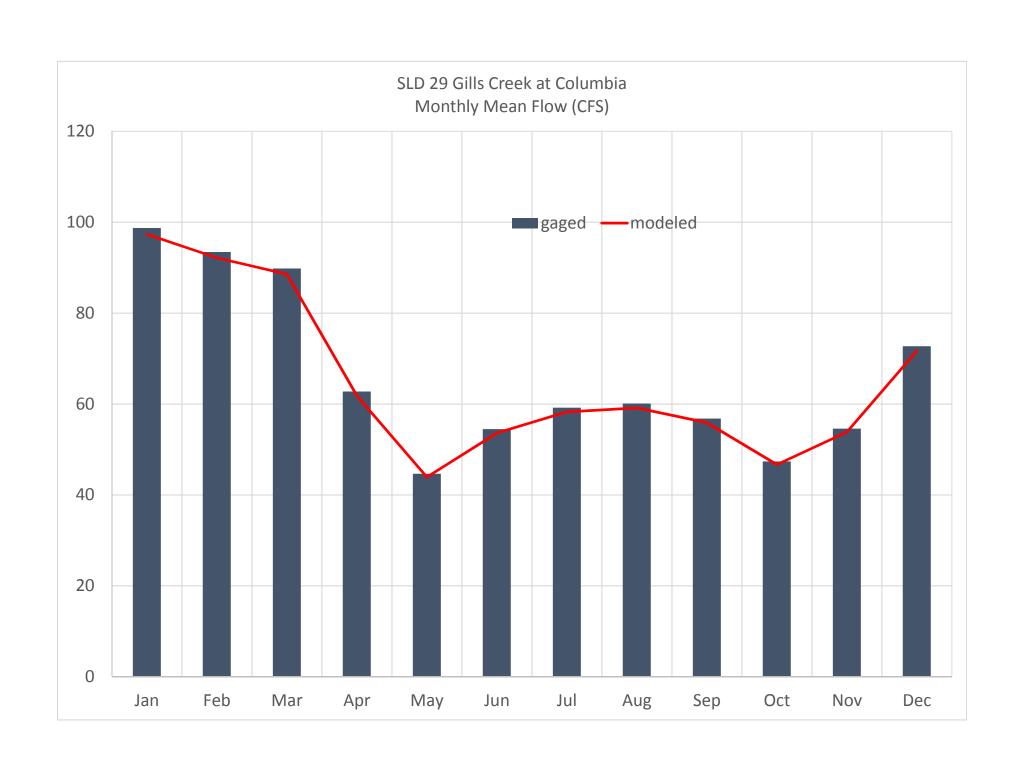


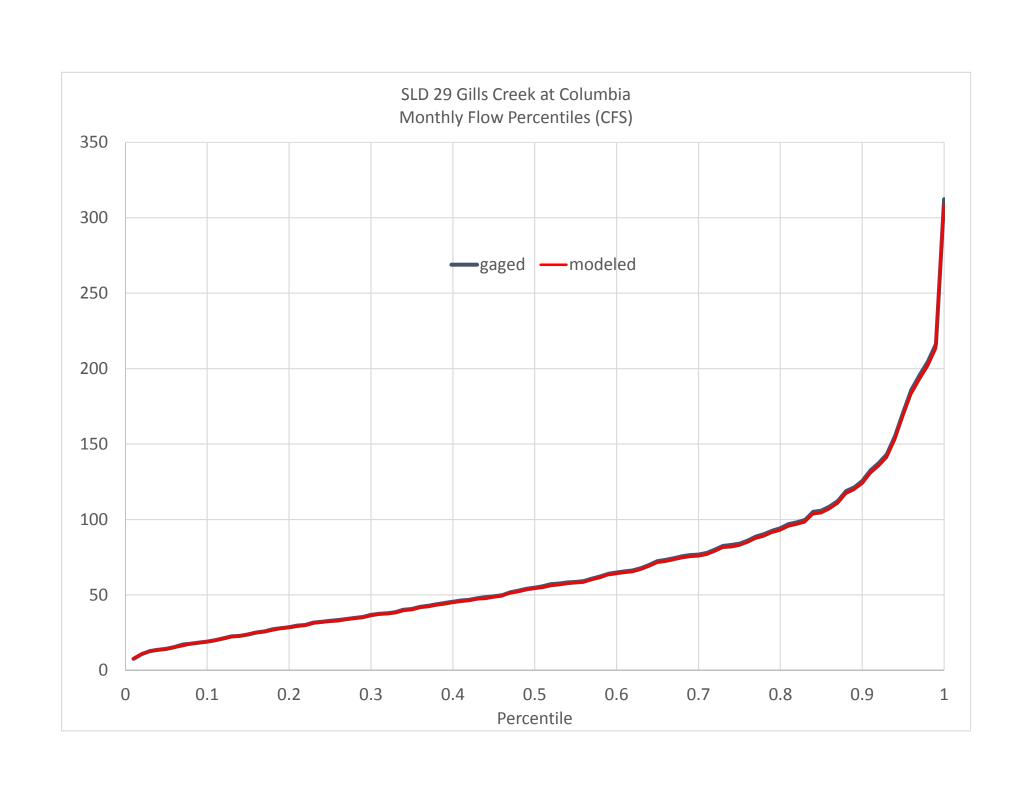


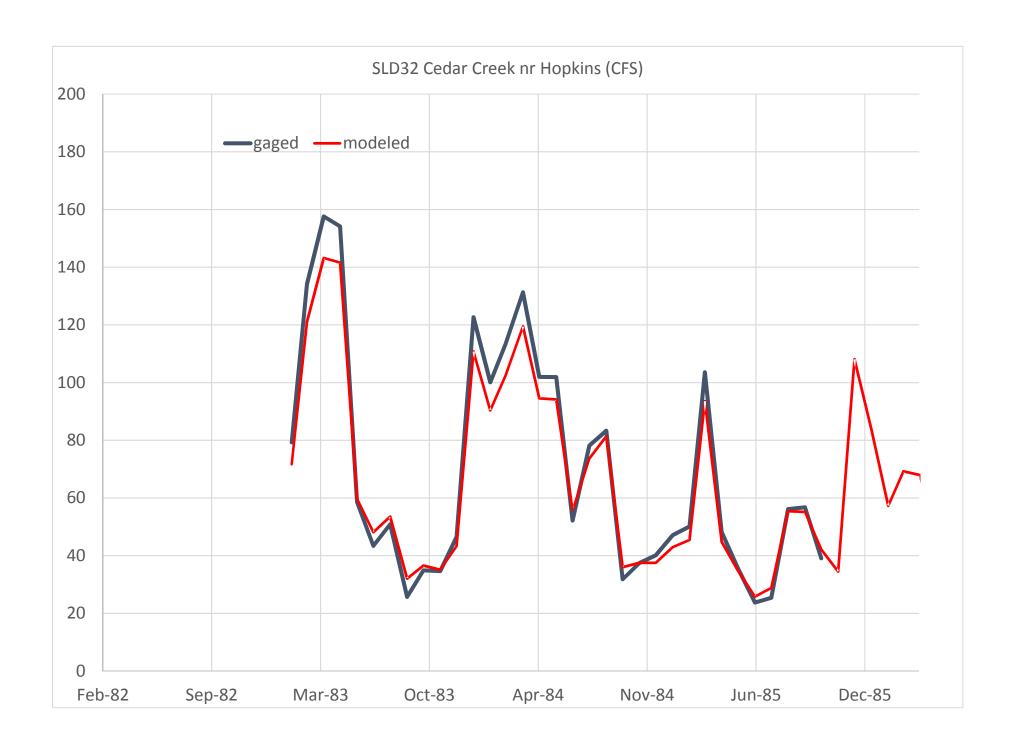


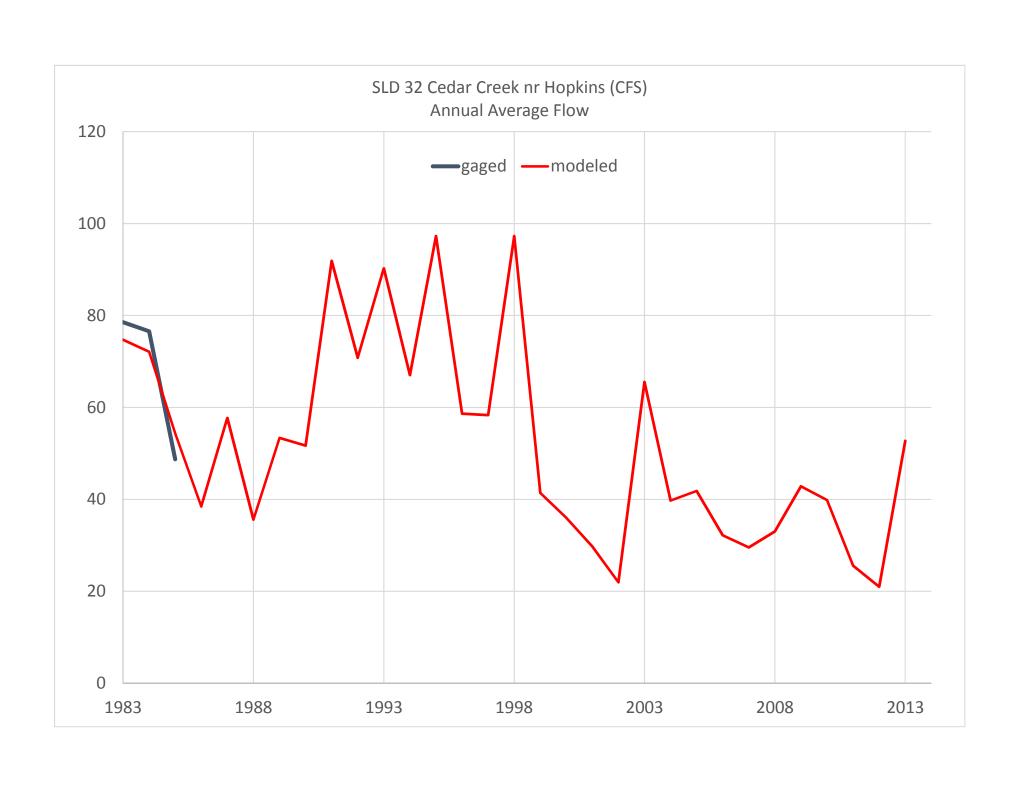


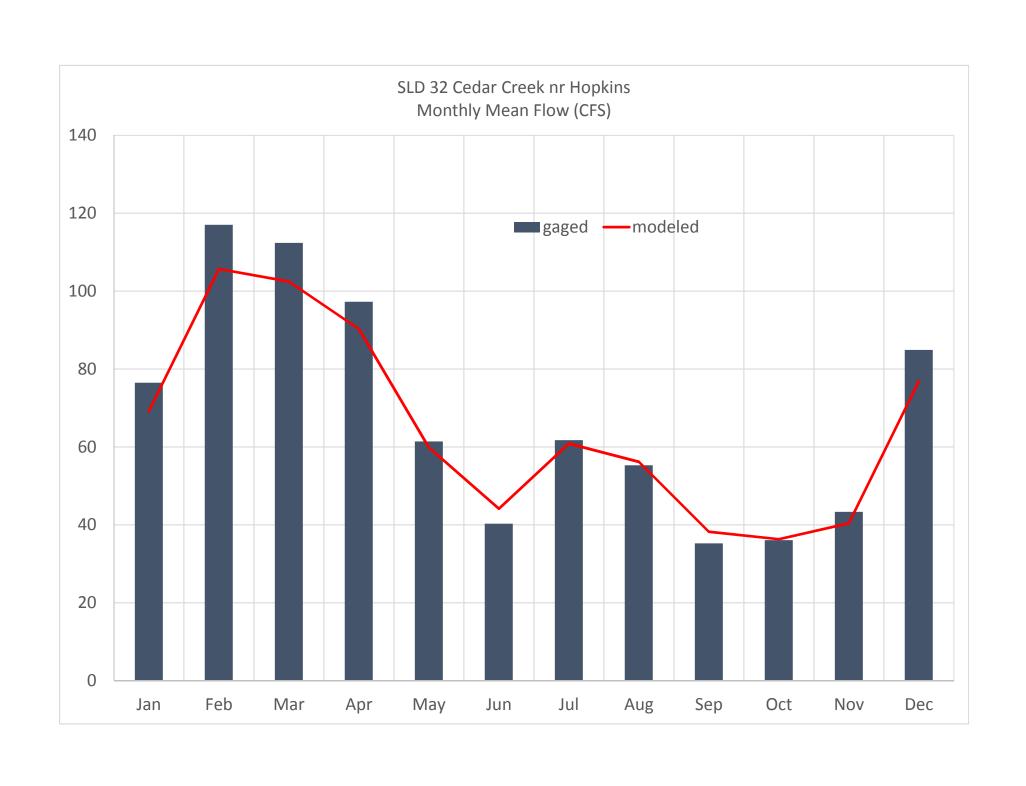


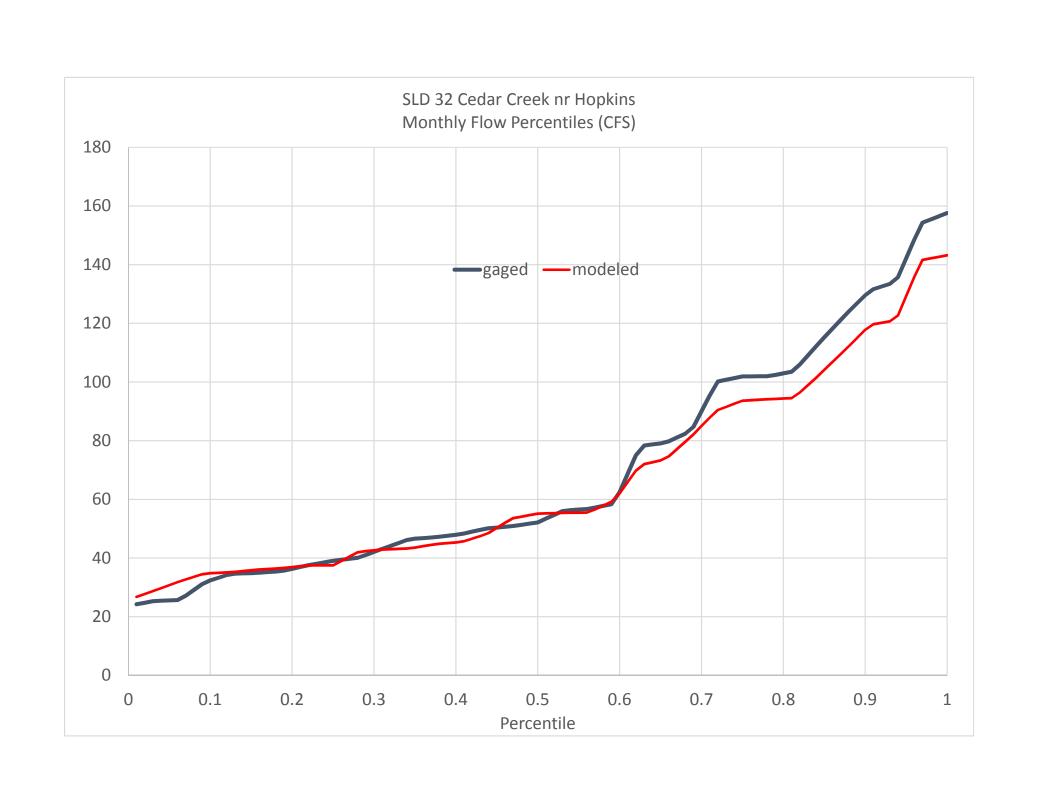


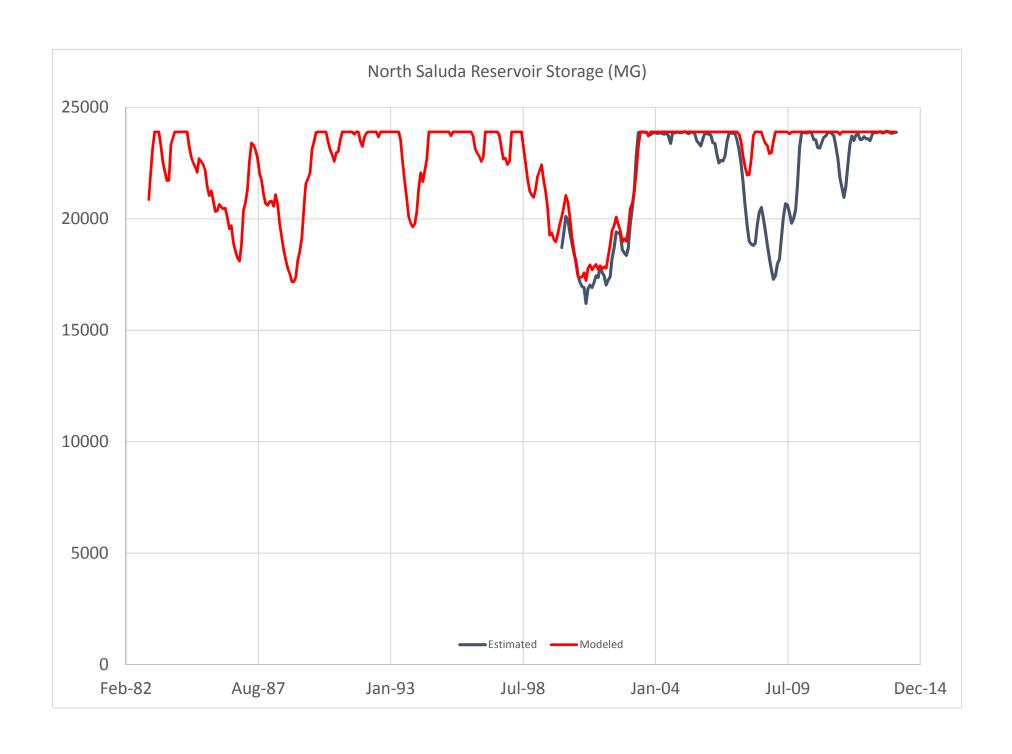


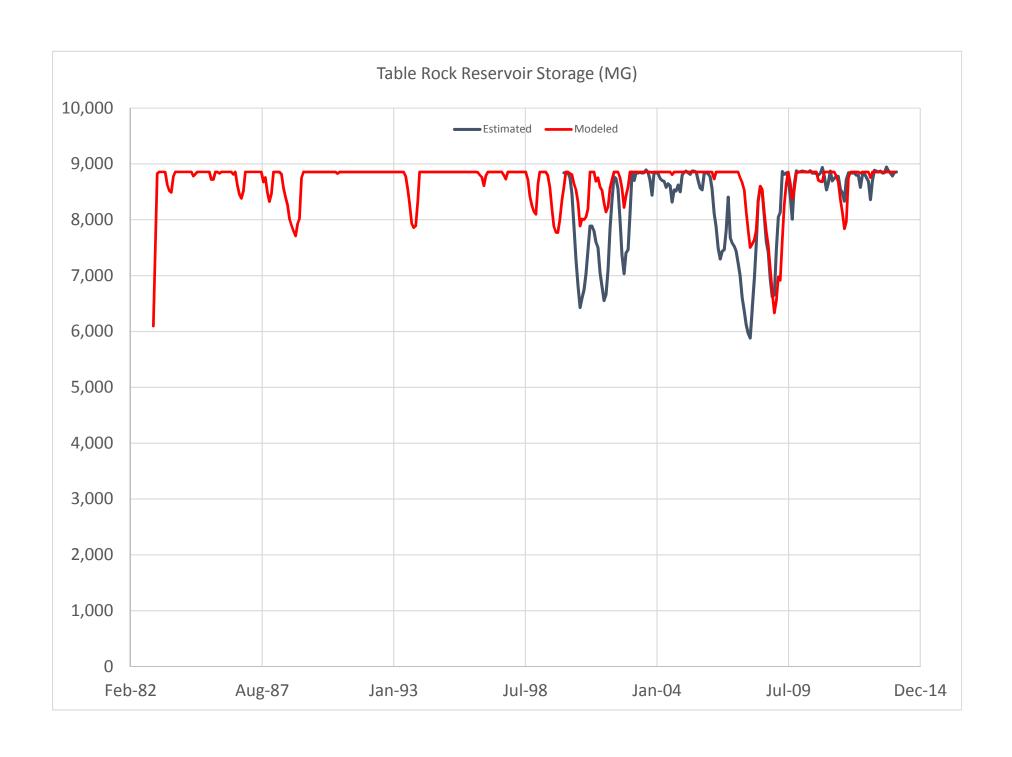


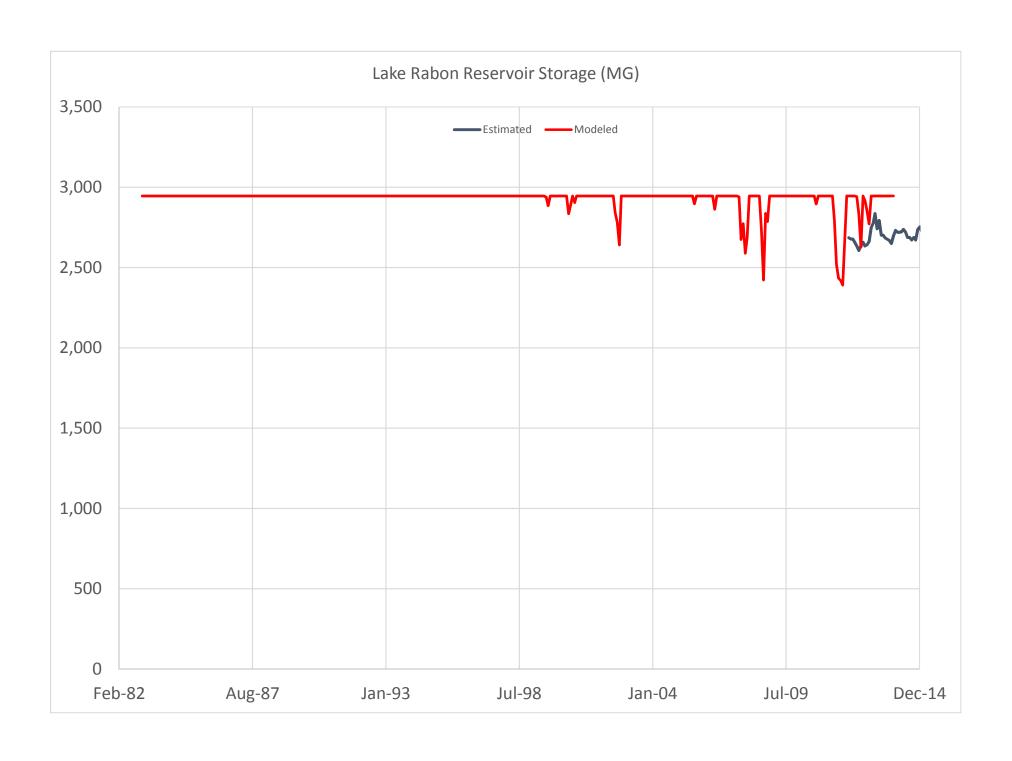


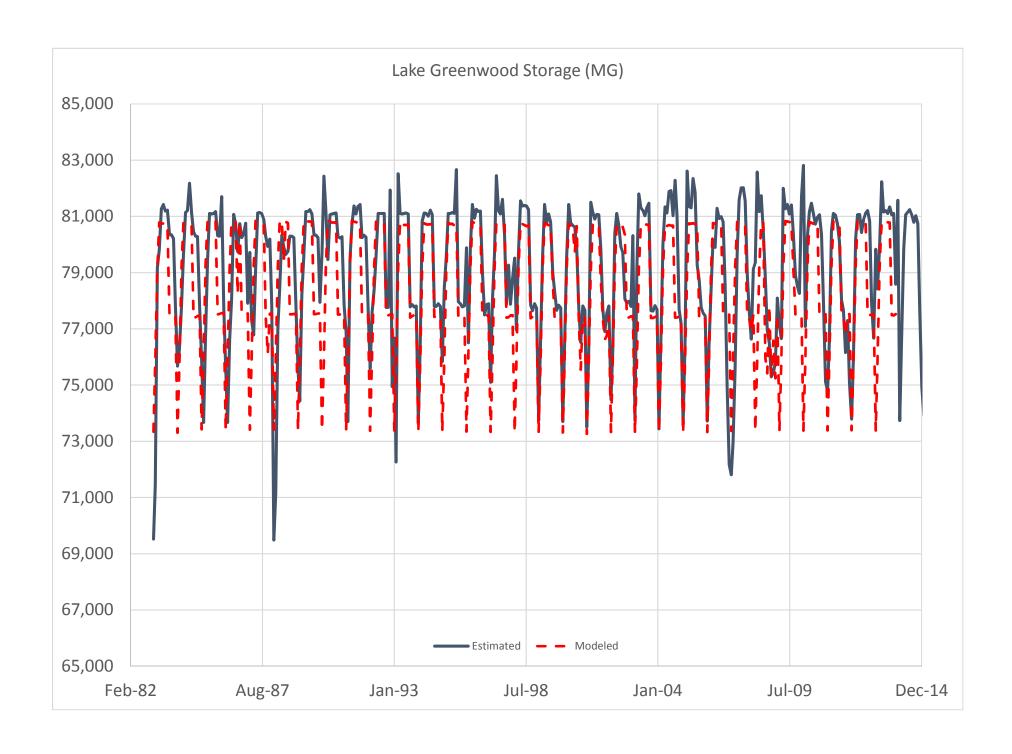


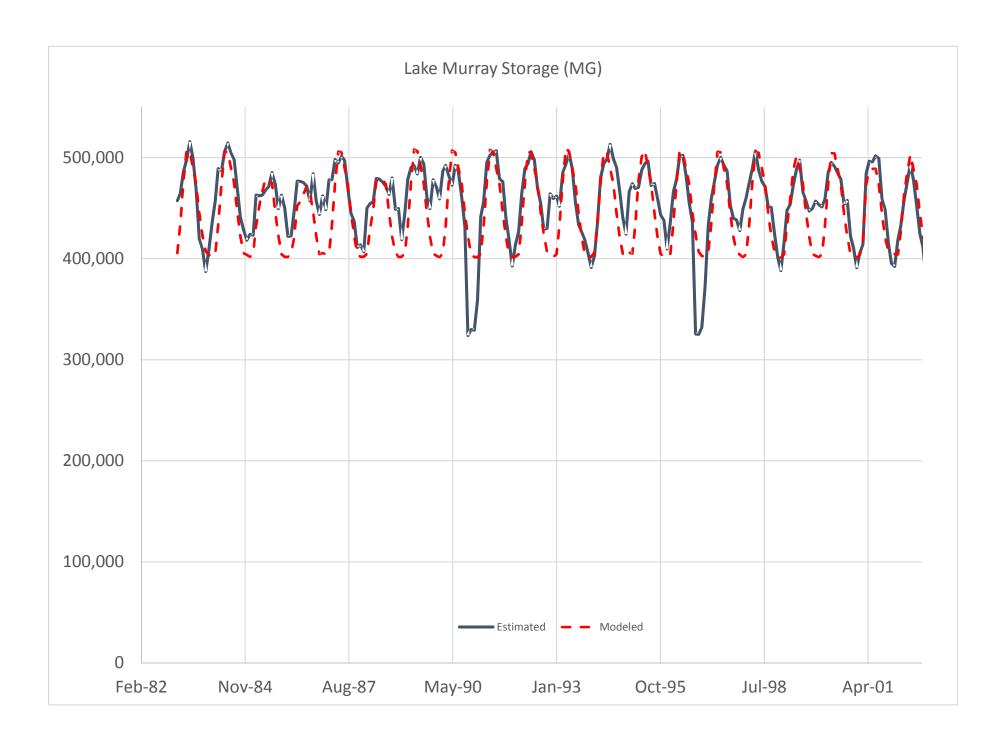










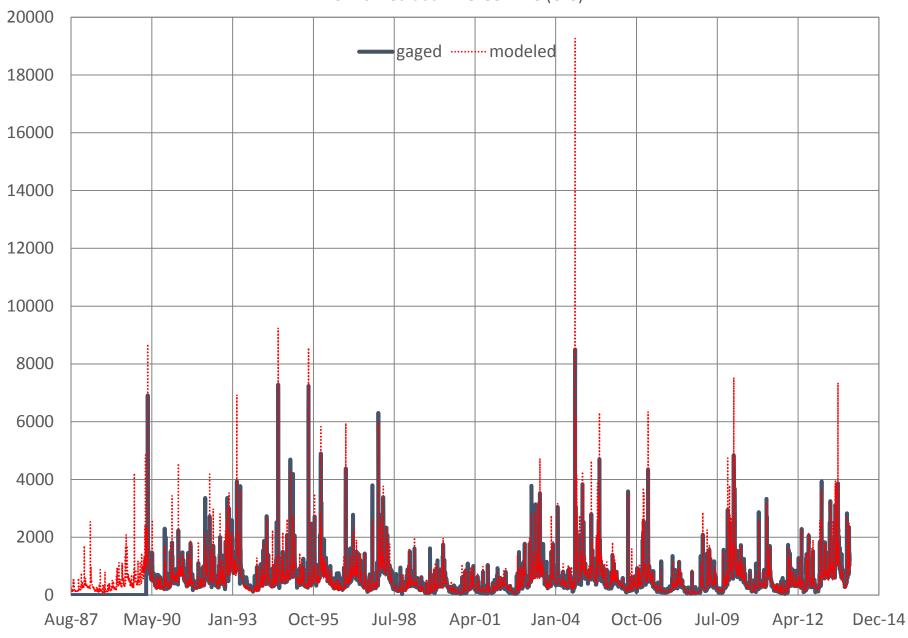


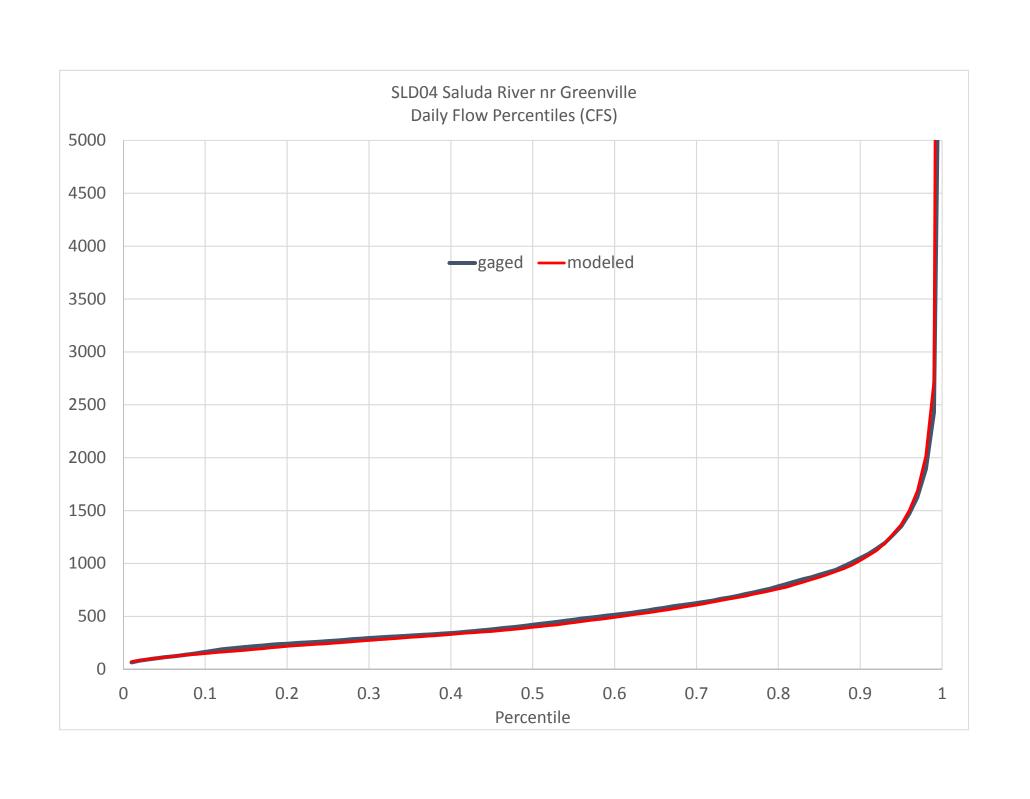
Appendix B

Saluda River Basin Model Daily Calibration Results

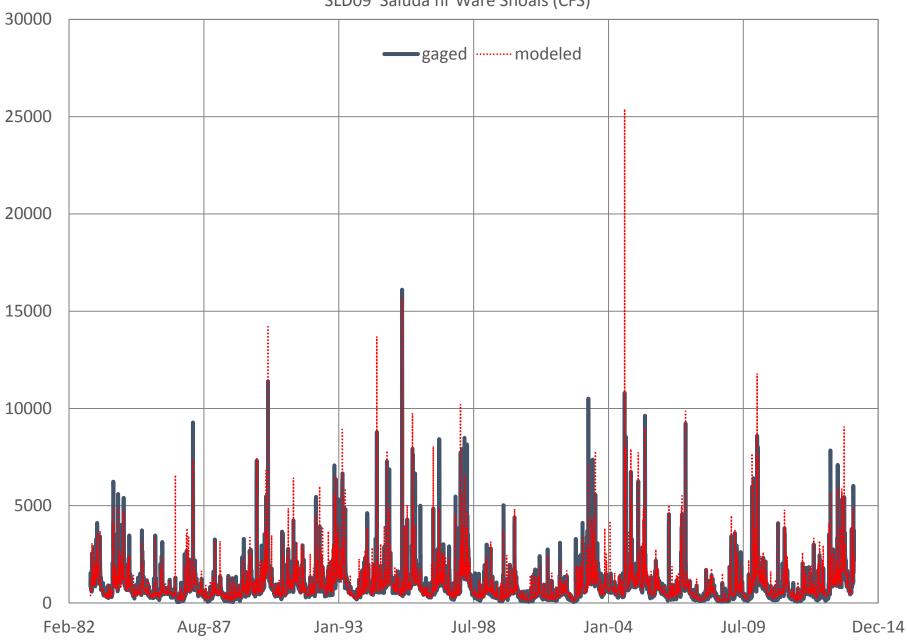


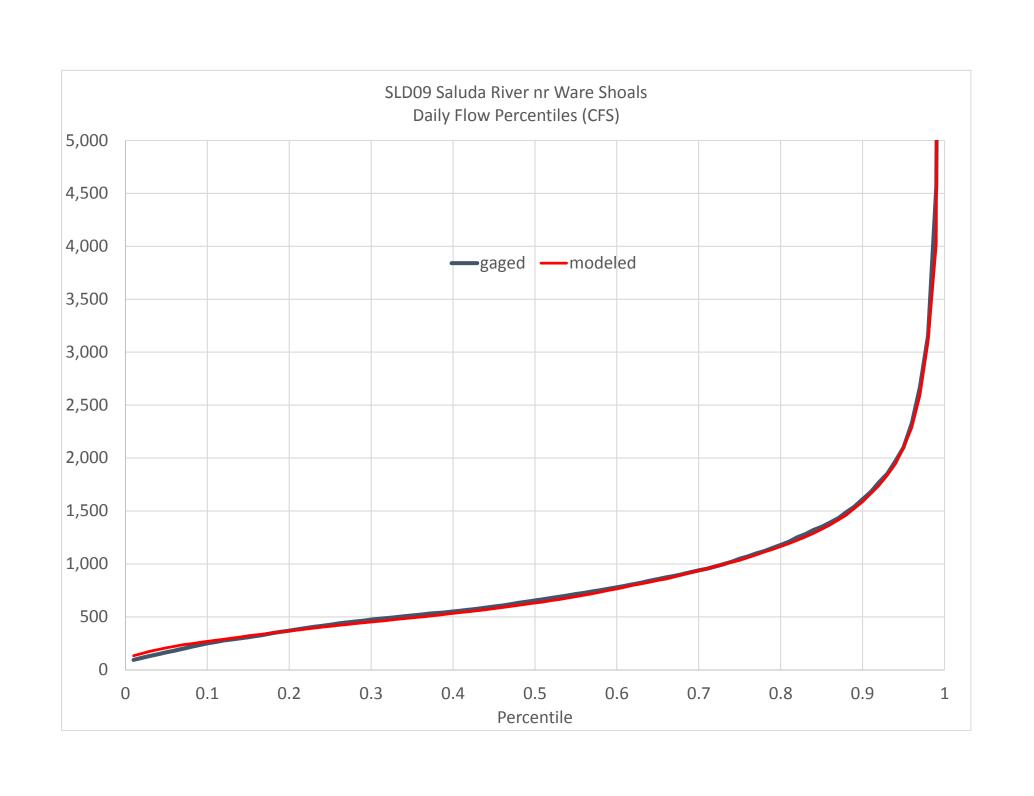
SLD04 Saluda nr Greenville (CFS)



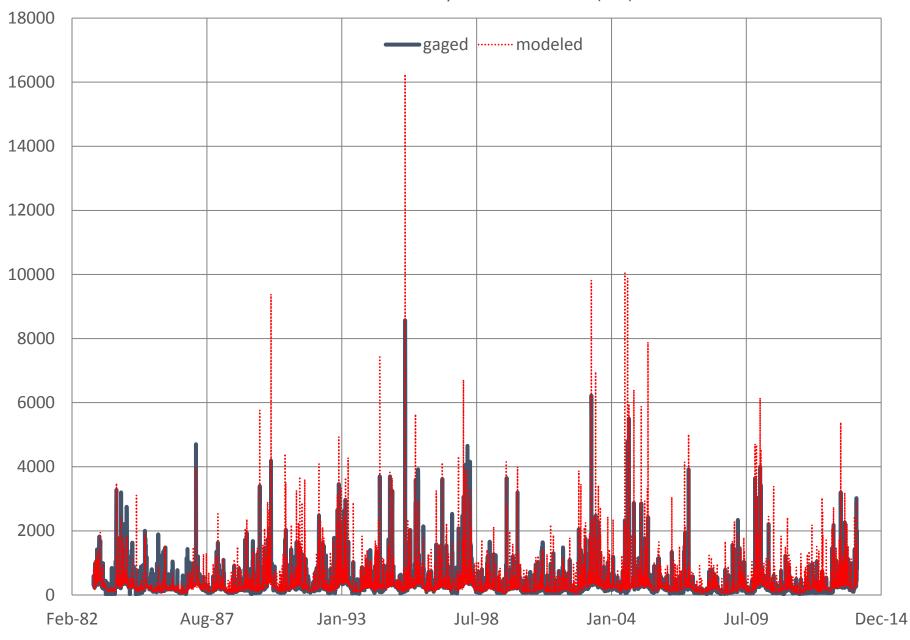


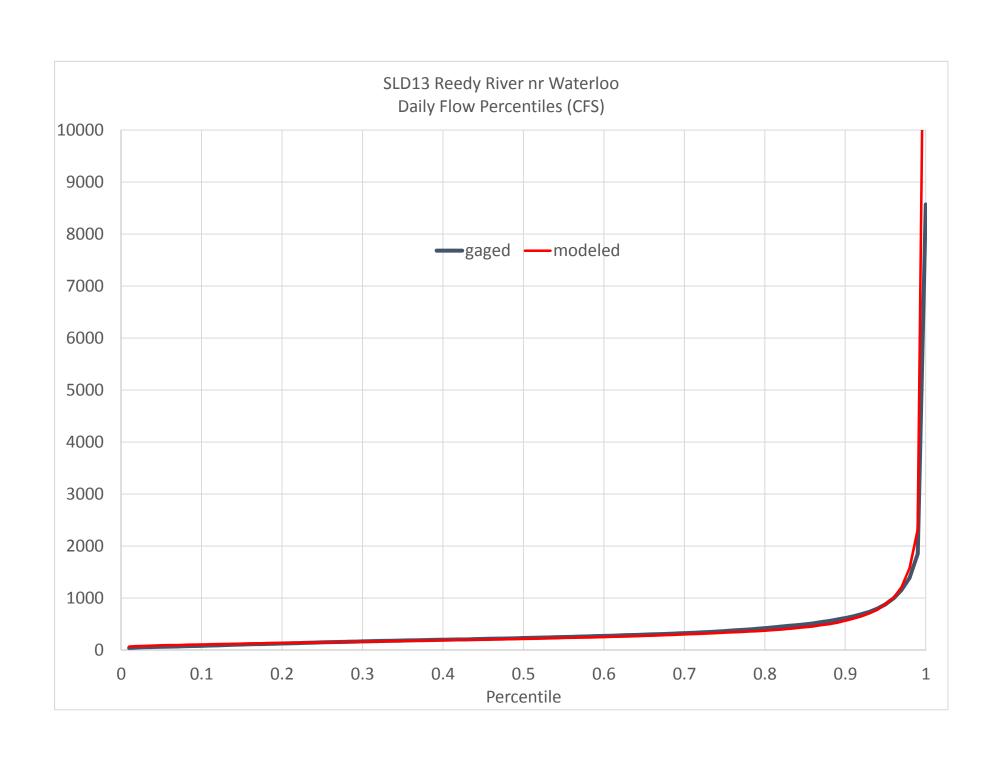




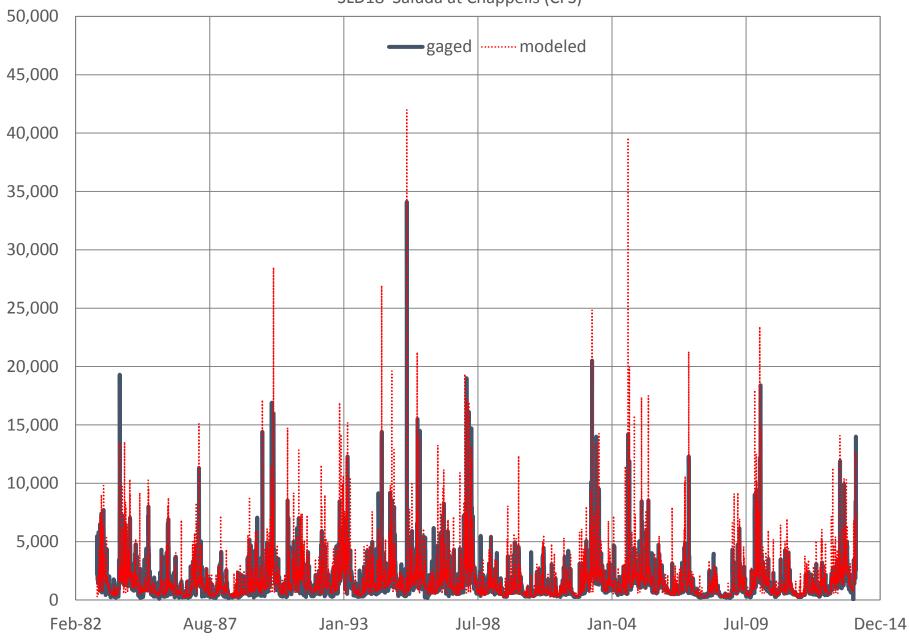


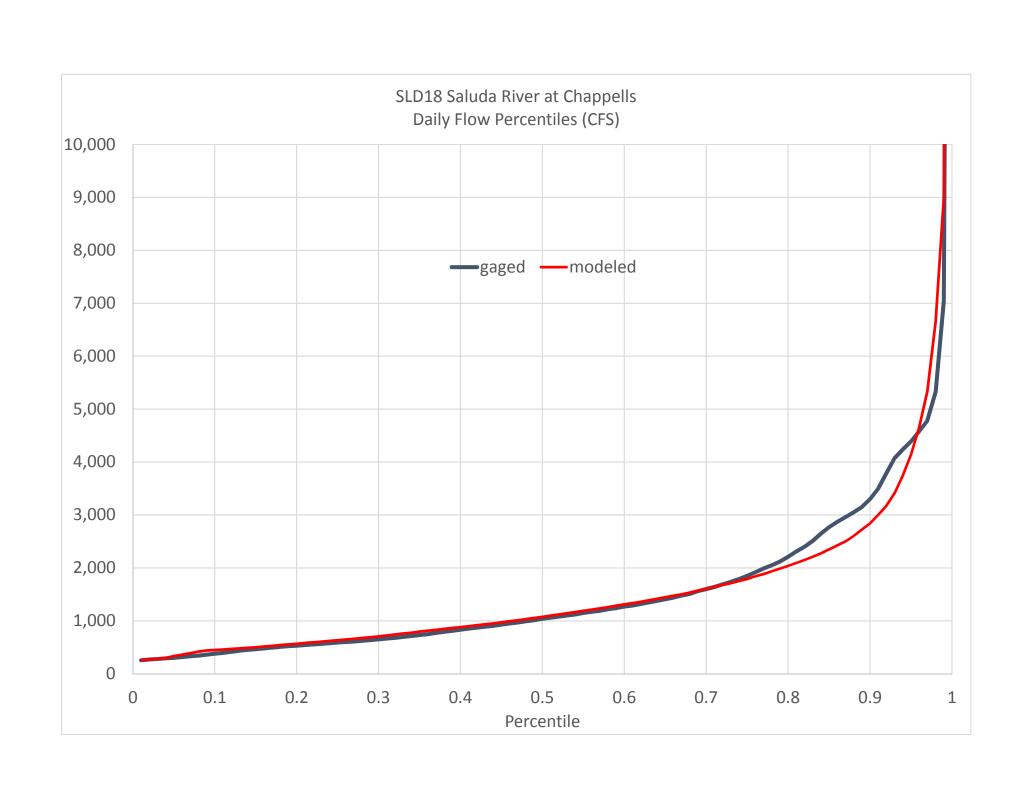
SLD12 & SLD13 Reedy River nr Waterloo (CFS)



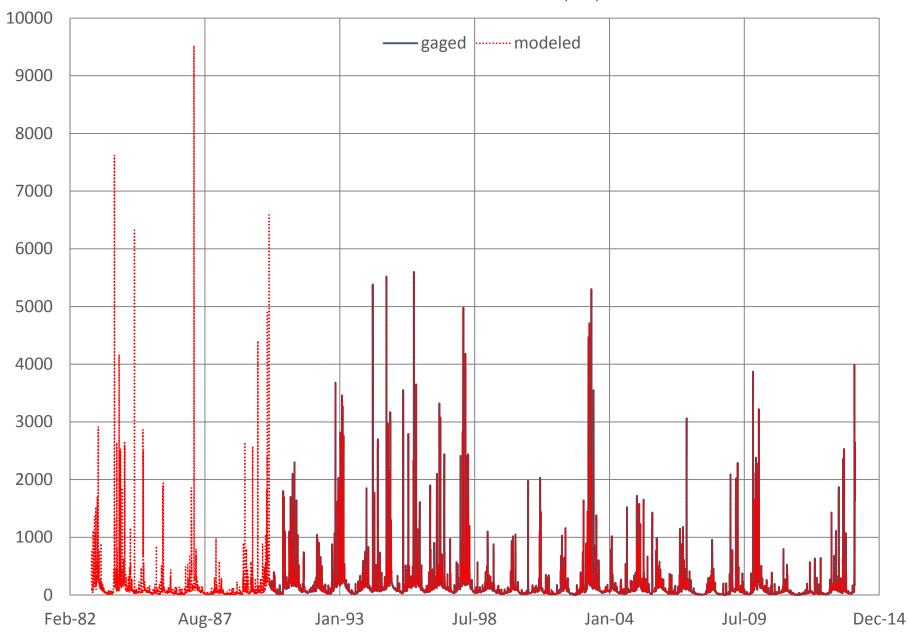


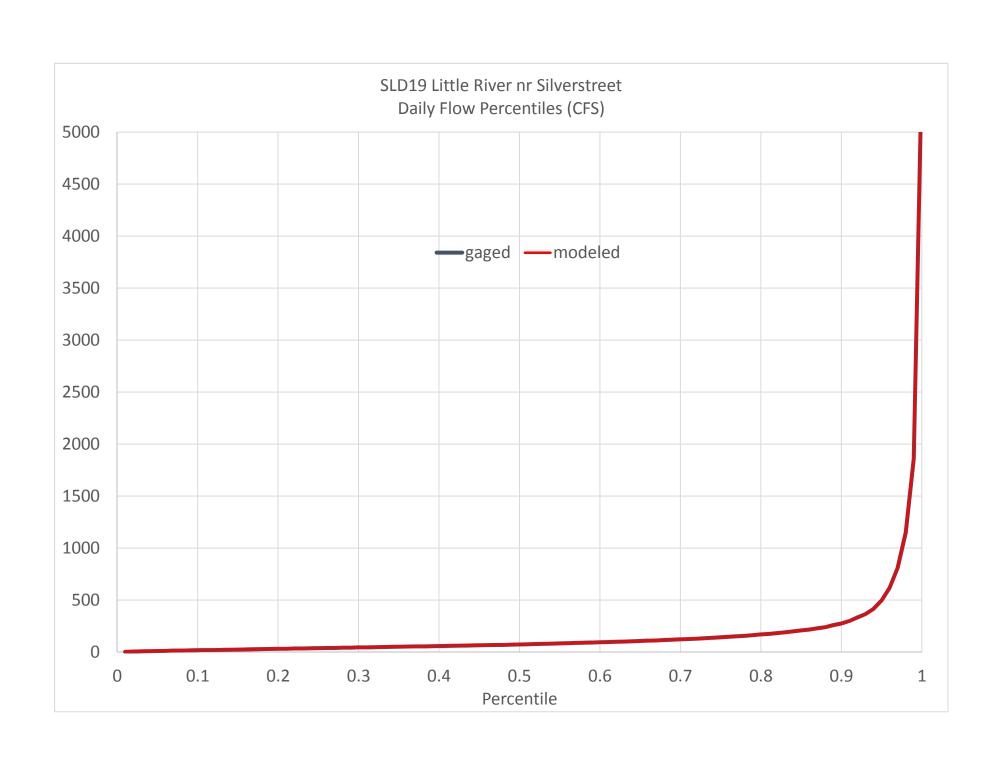
SLD18 Saluda at Chappells (CFS)



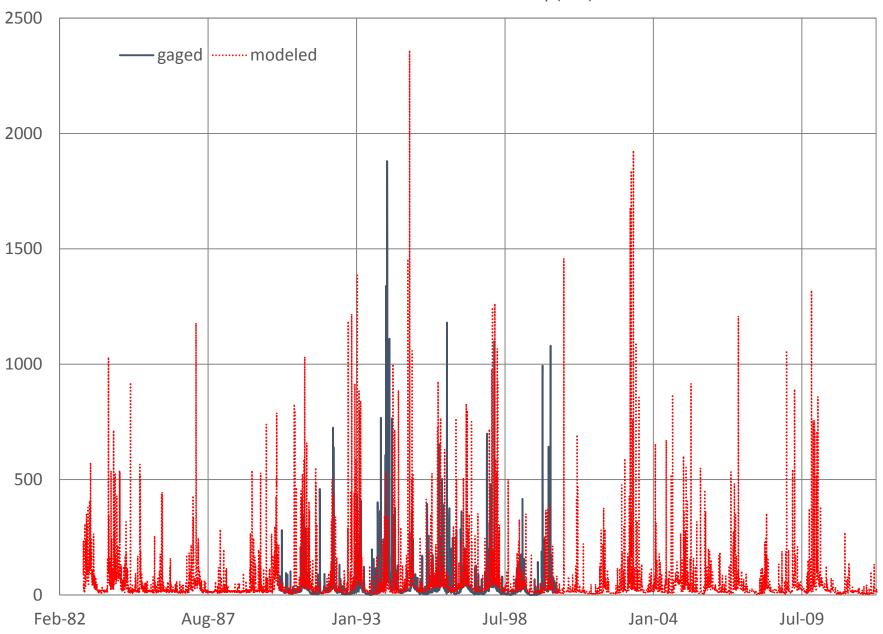


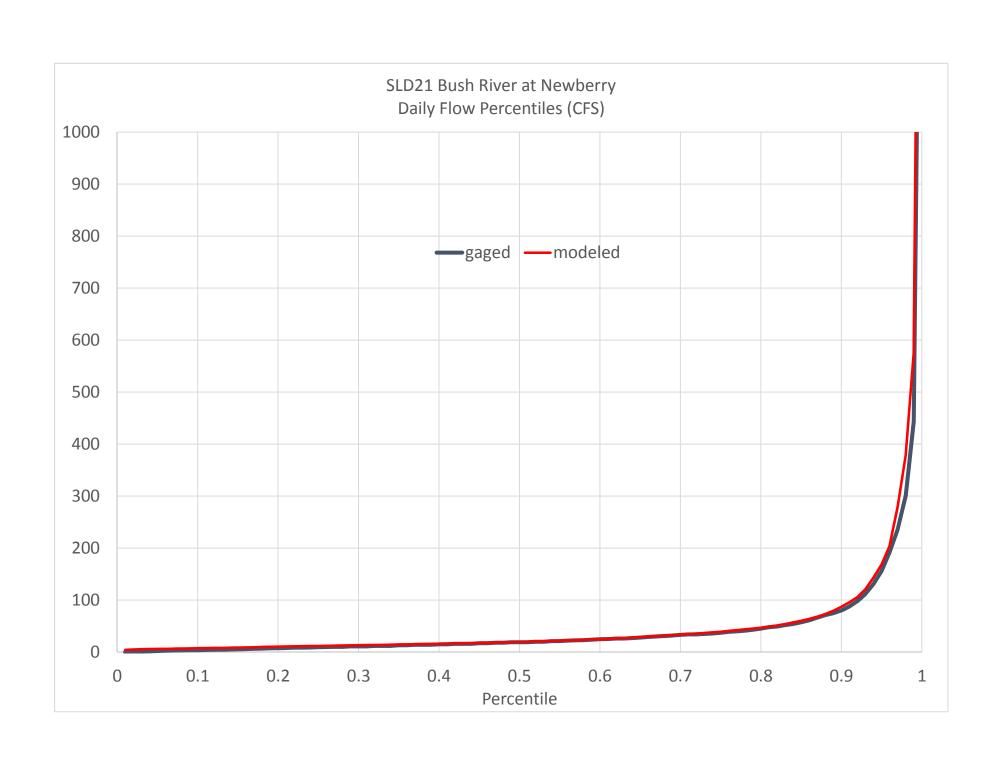
SLD19 Little River nr Silverstreet (CFS)



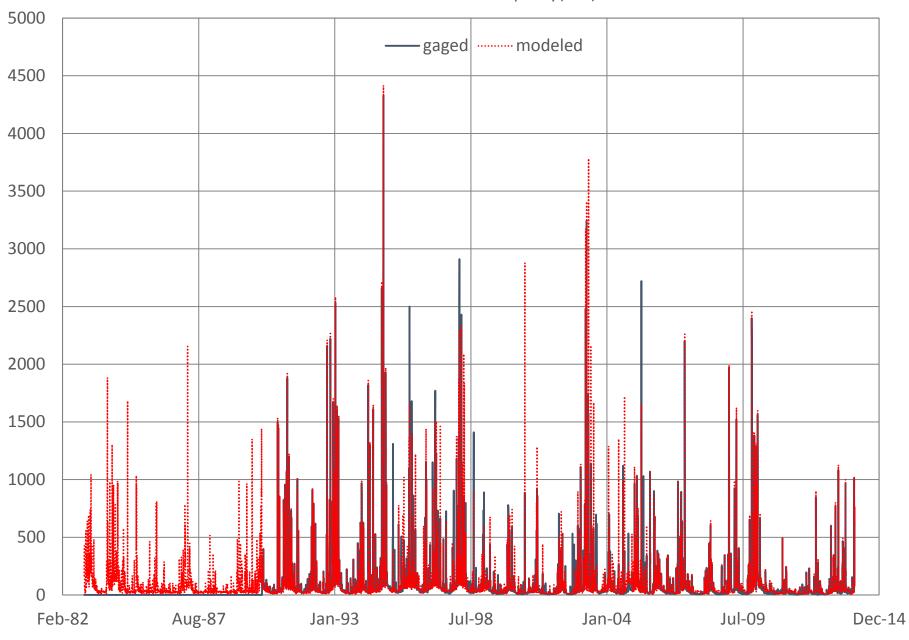


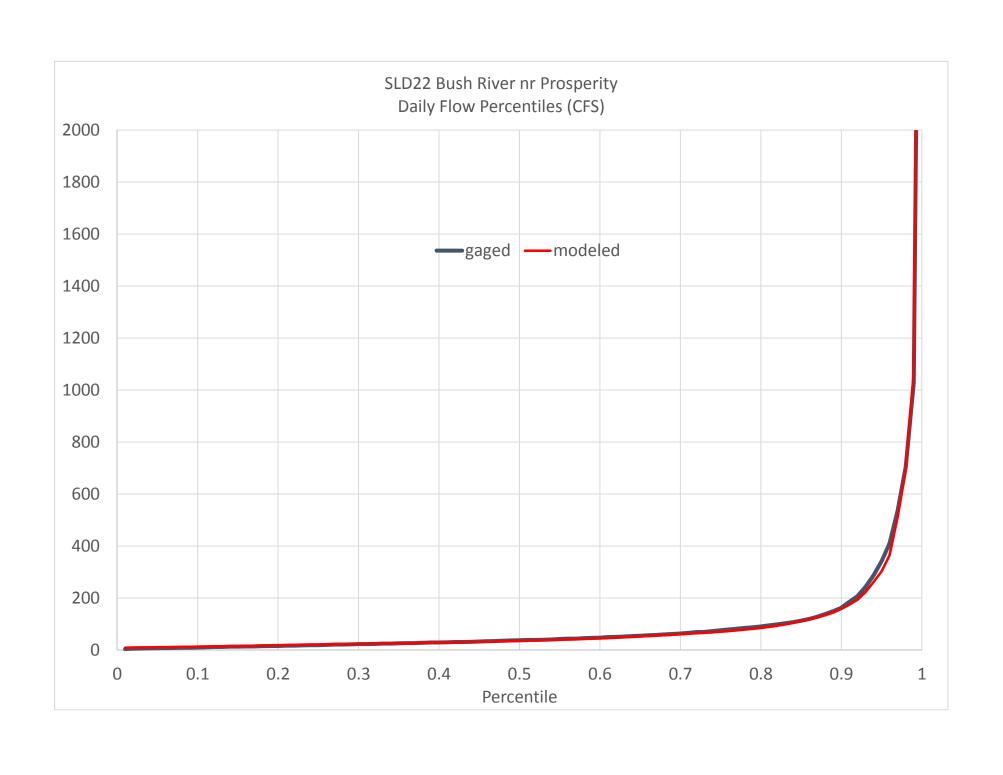
SLD21 Bush River at Newberry (CFS)



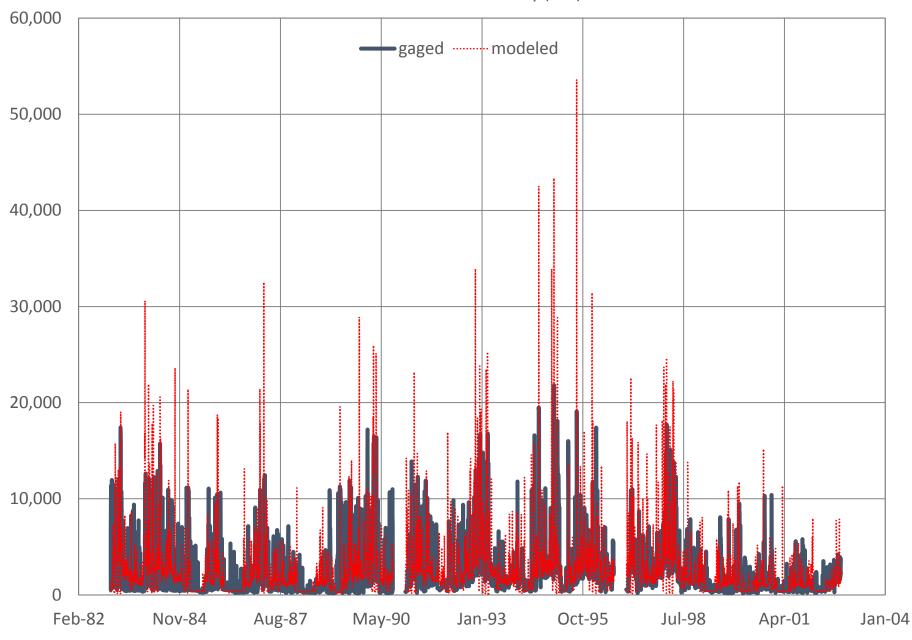


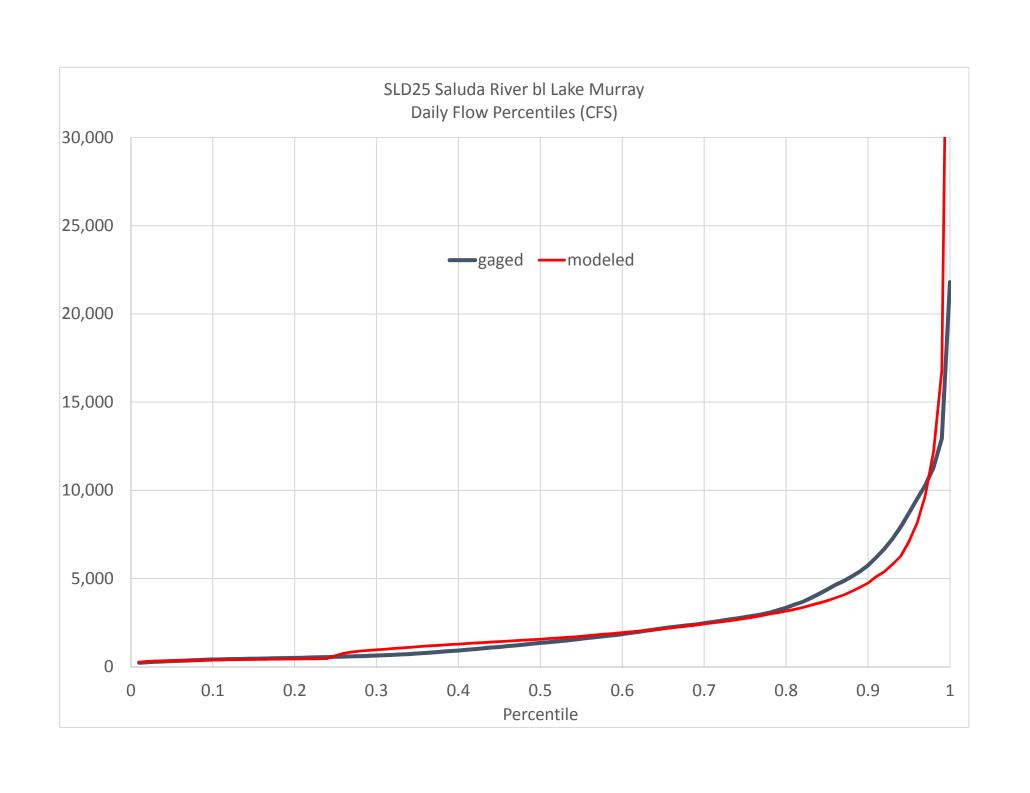
SLD22 Bush River nr Prosperity(CFS)



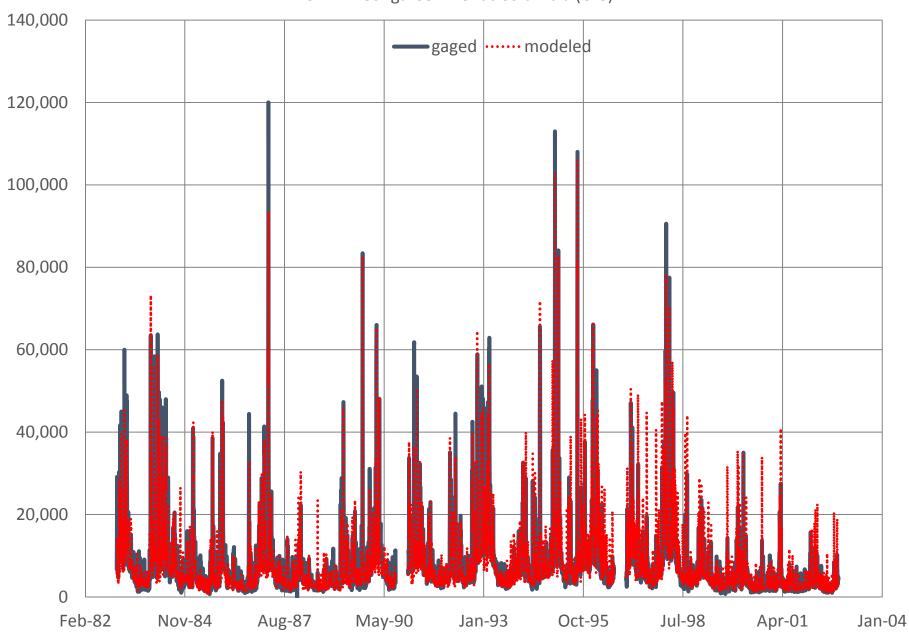


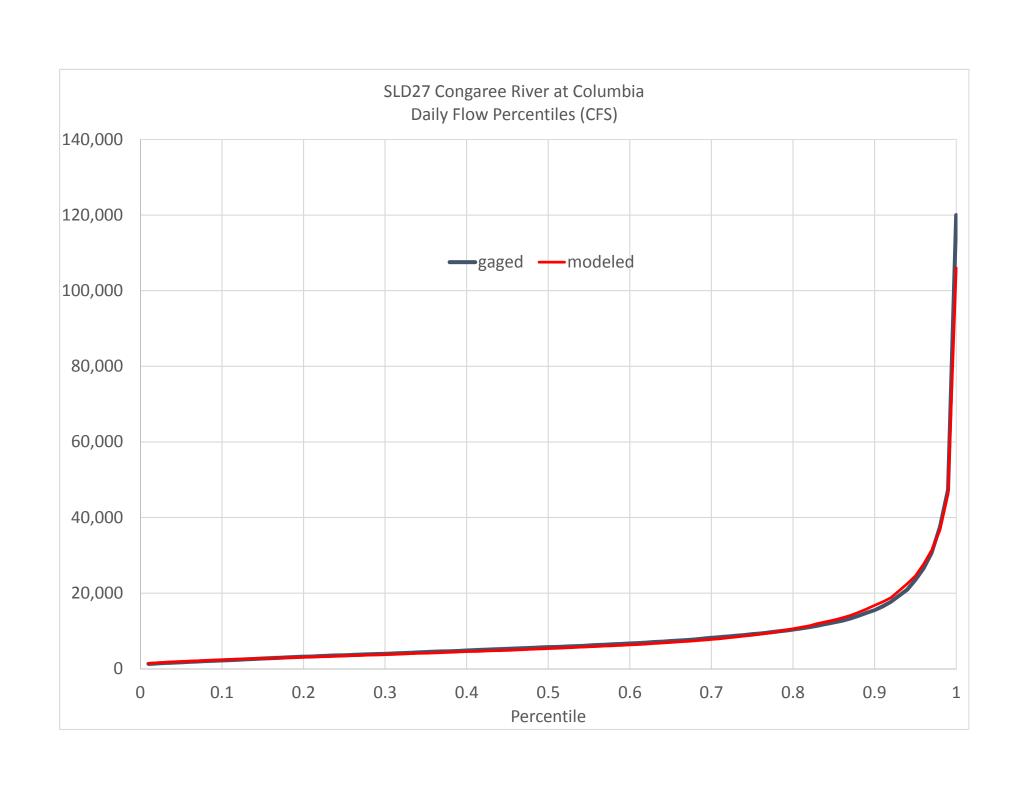
SLD25 Below Lake Murray (CFS)



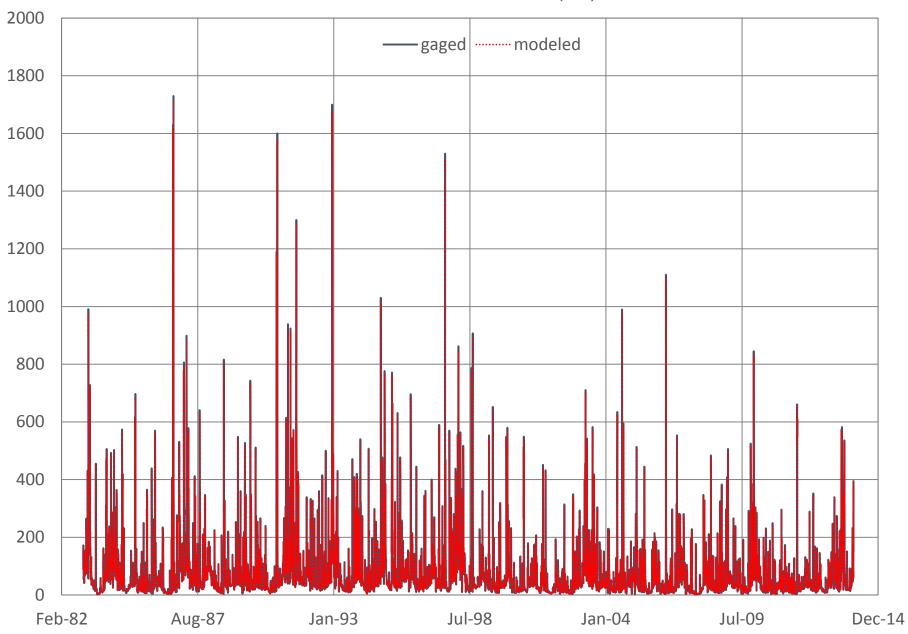


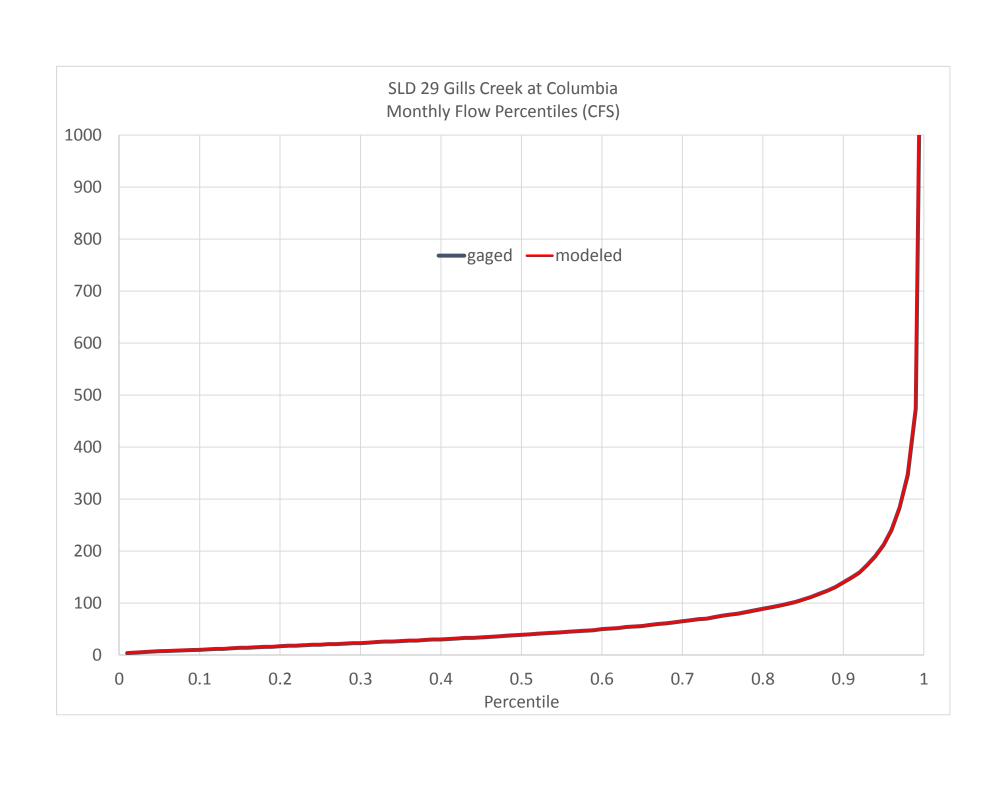
SLD 27 Congaree River at Columbia (CFS)



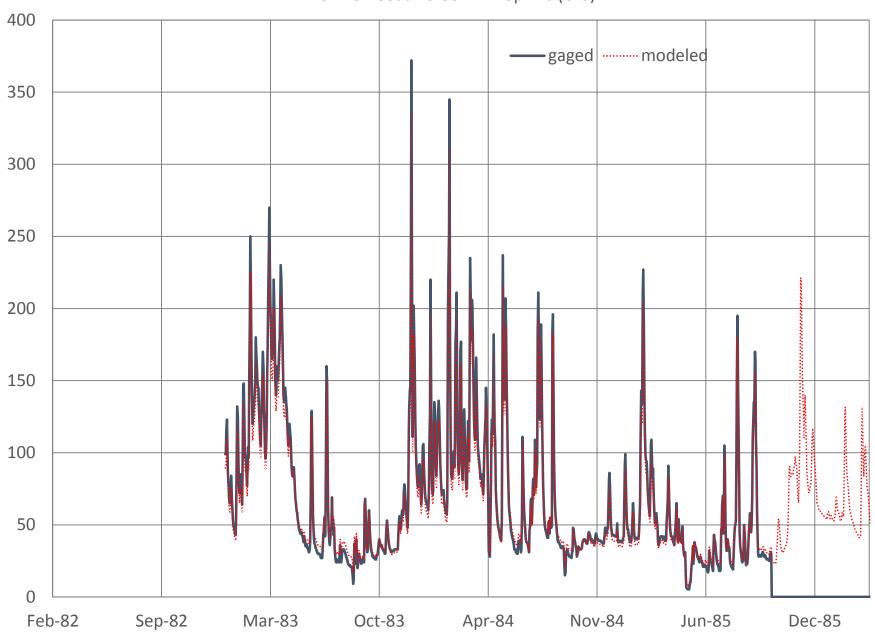


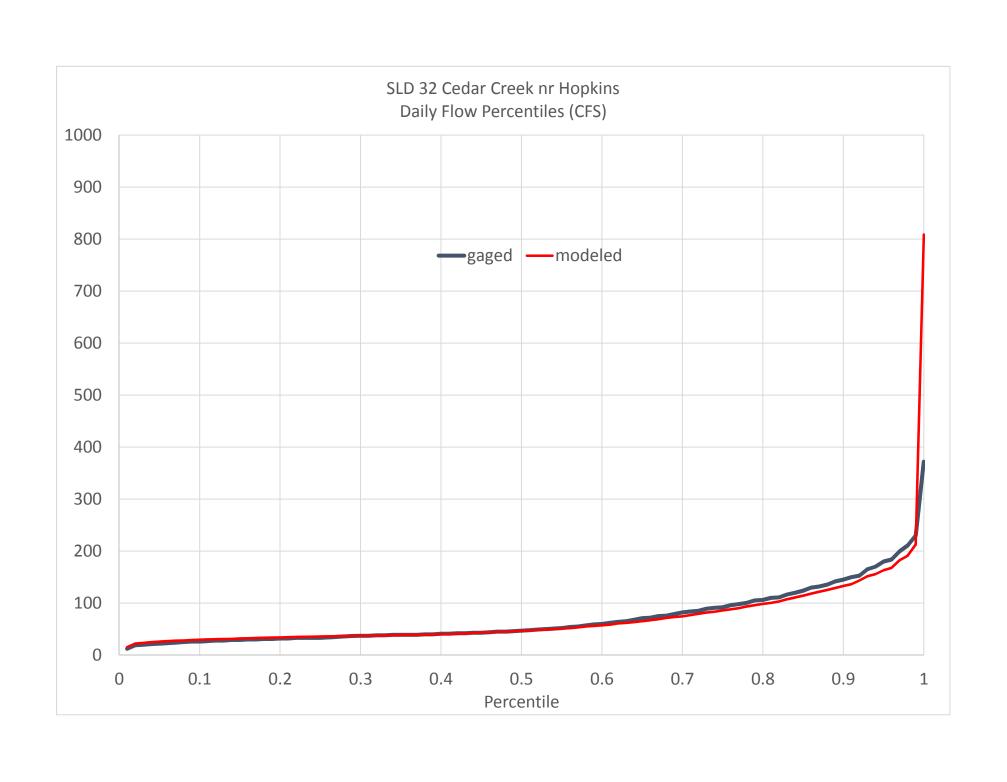
SLD 29 Gills Creek at Columbia (CFS)

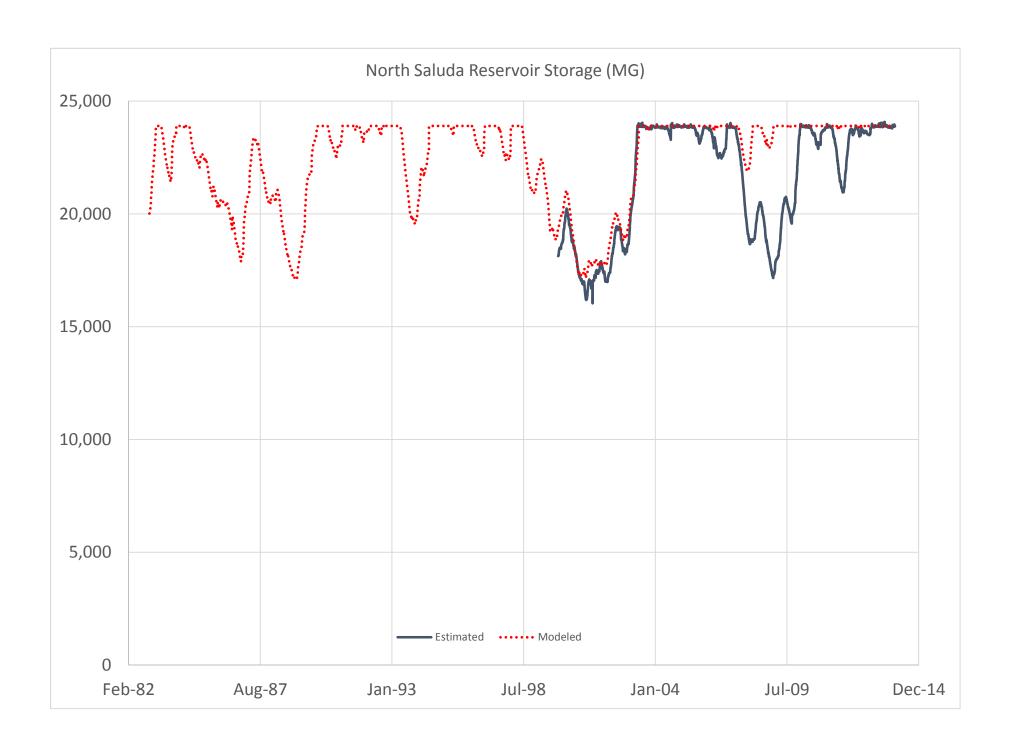


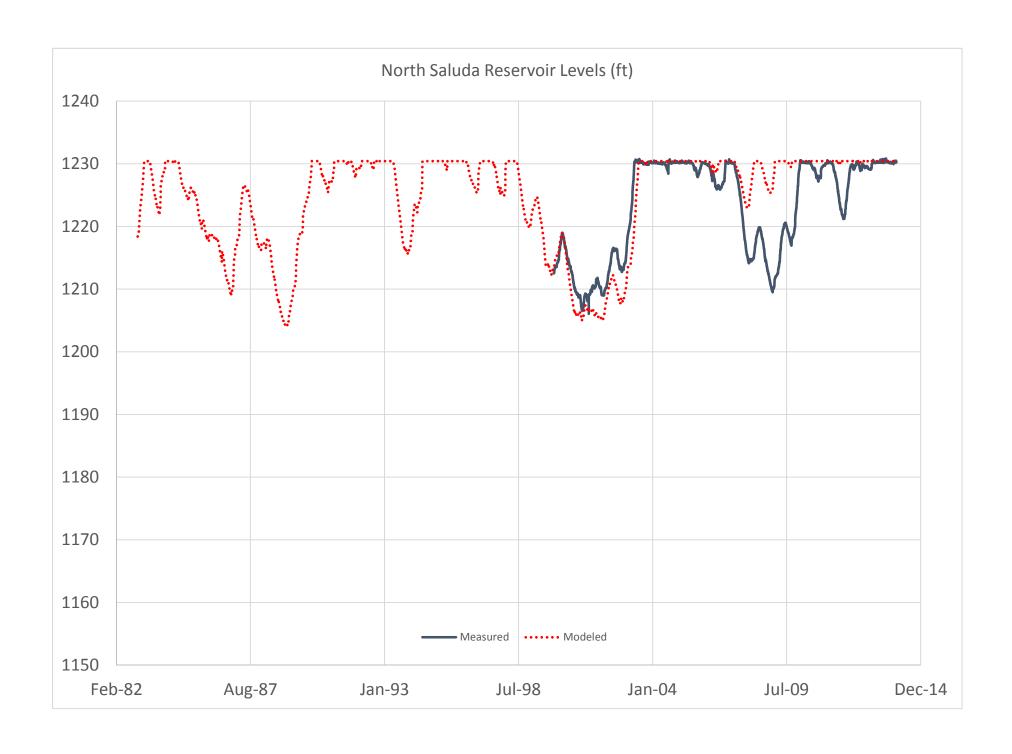


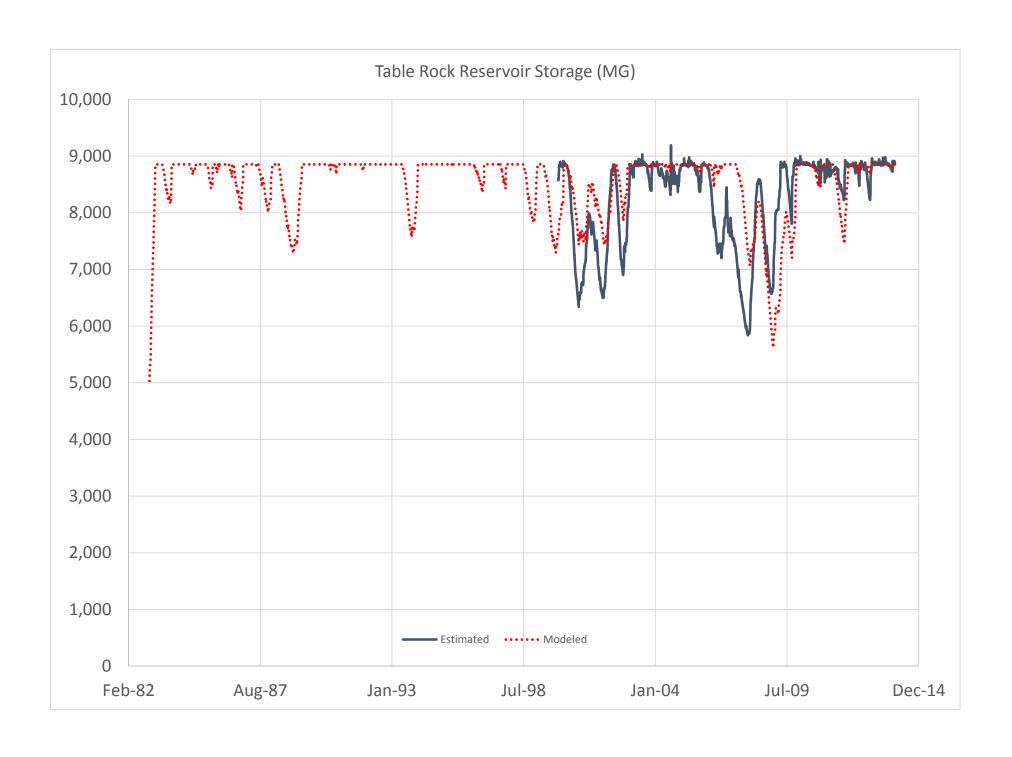
SLD 32 Cedar Creek nr Hopkins (CFS)

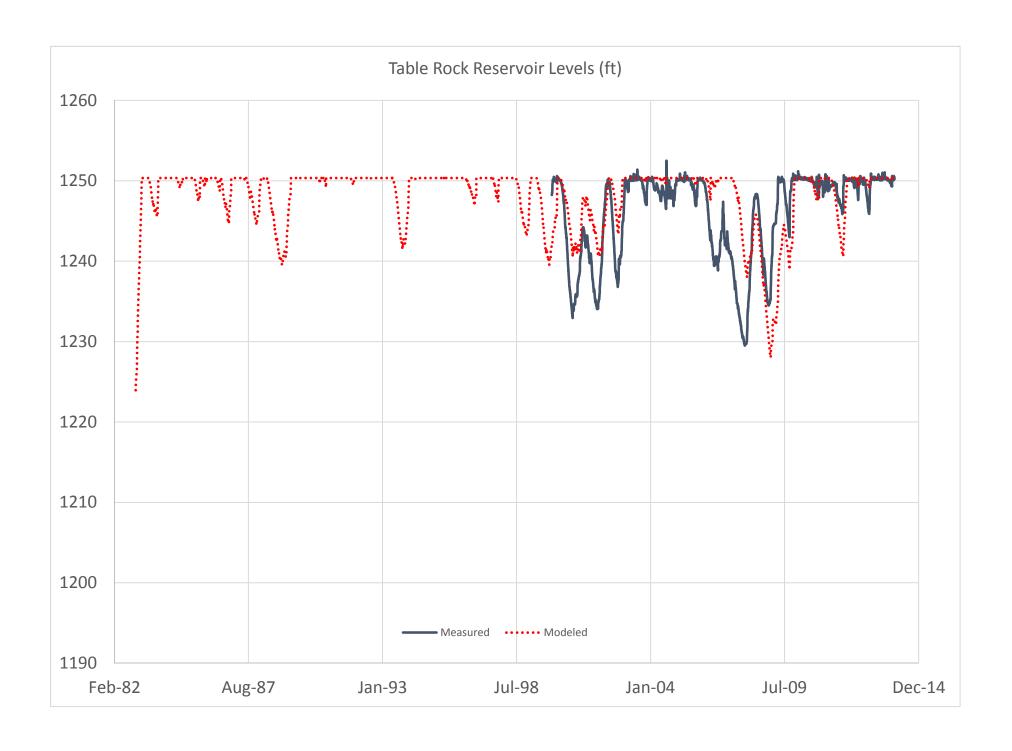


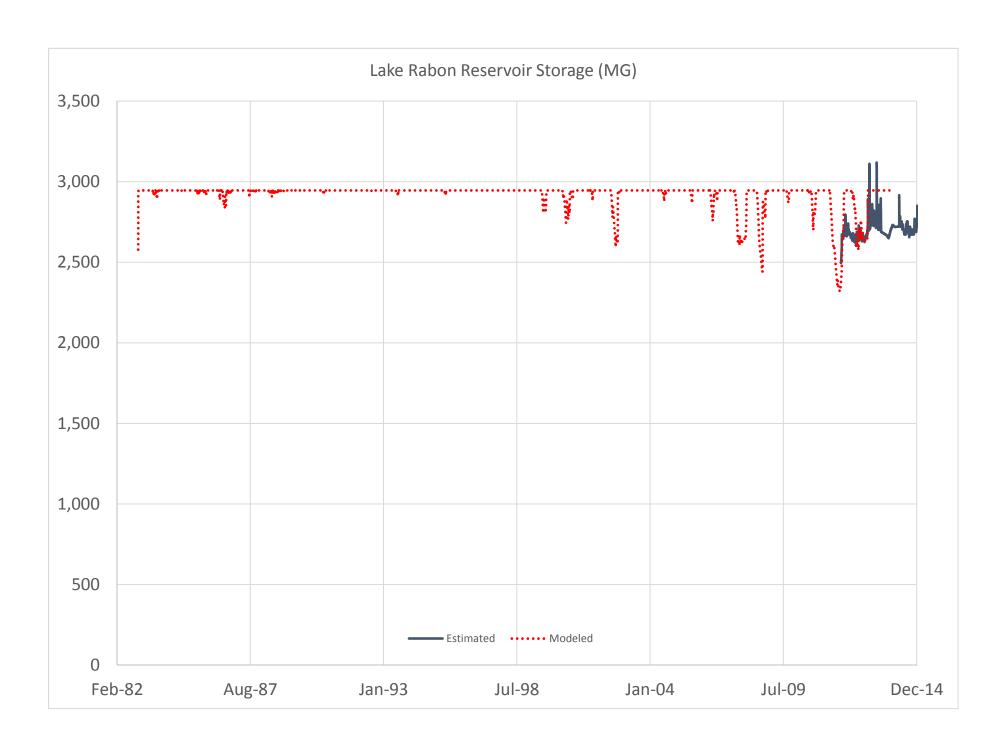


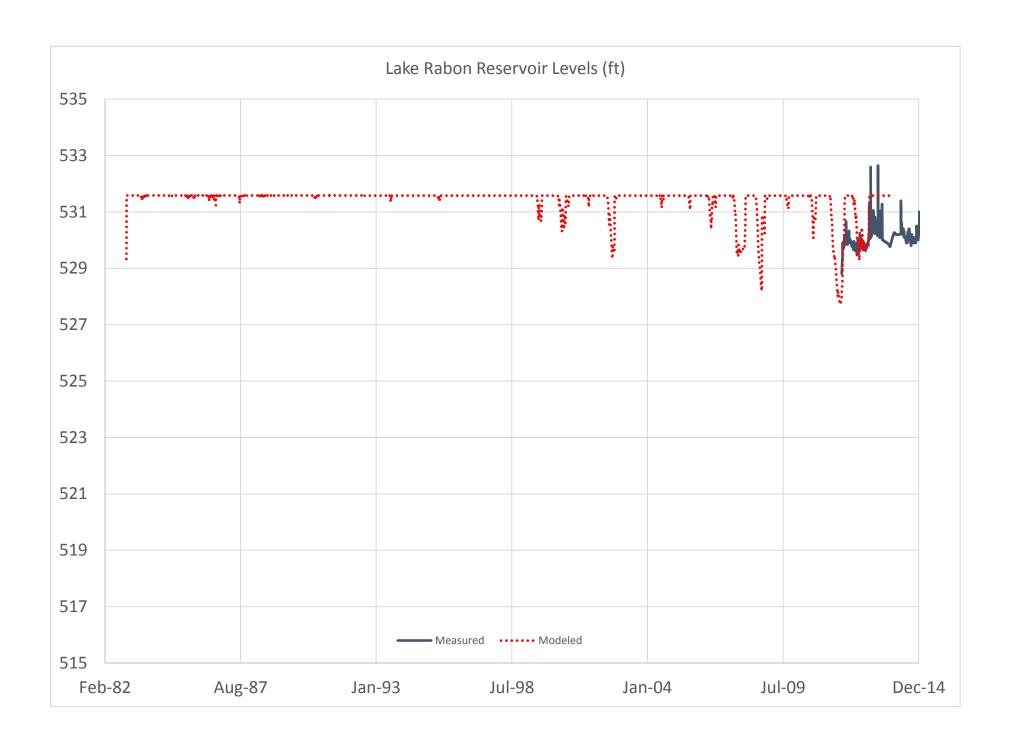


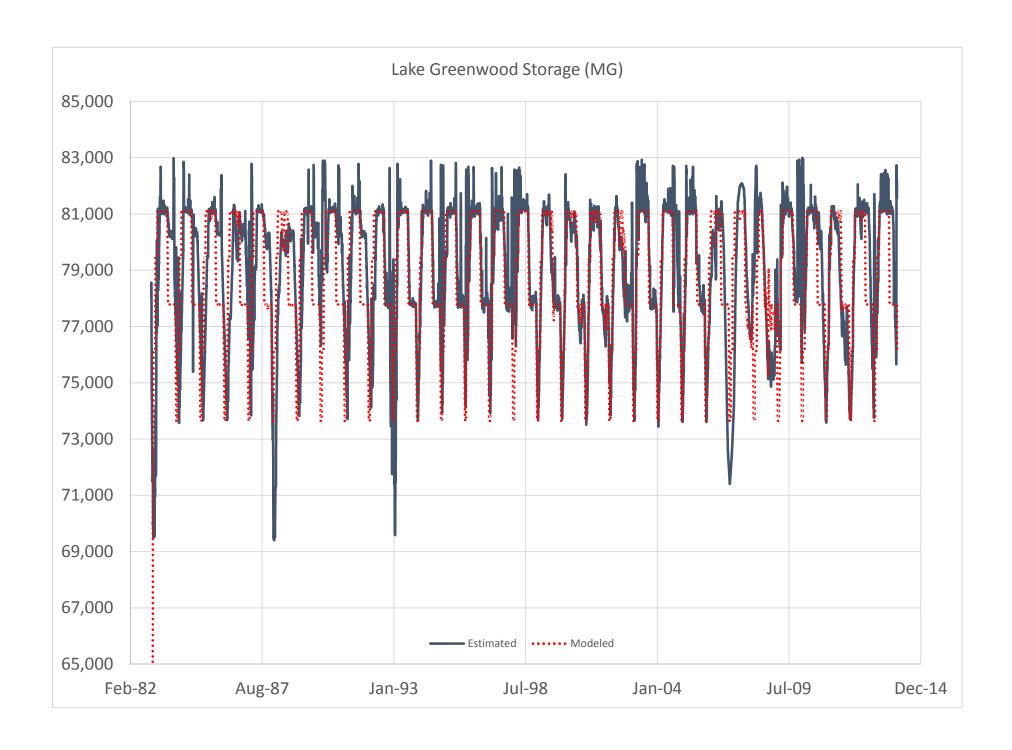




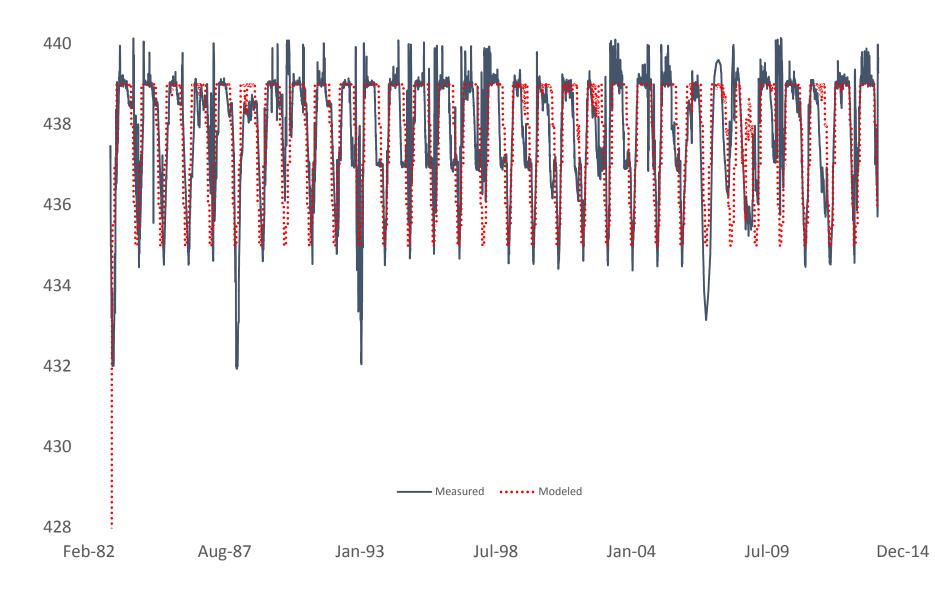


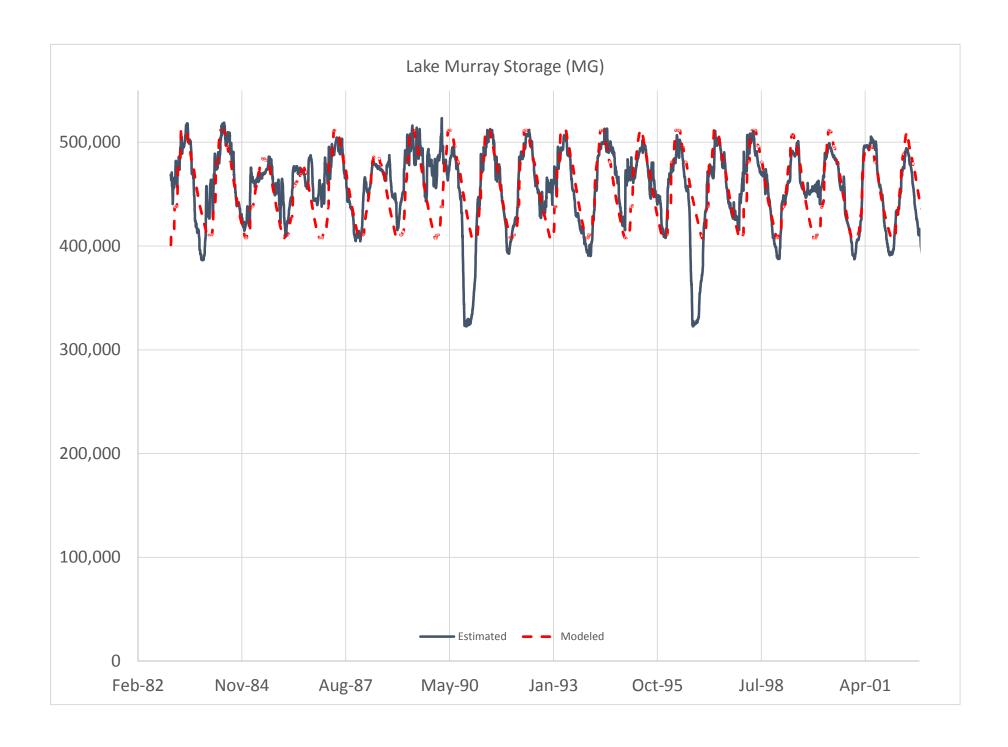


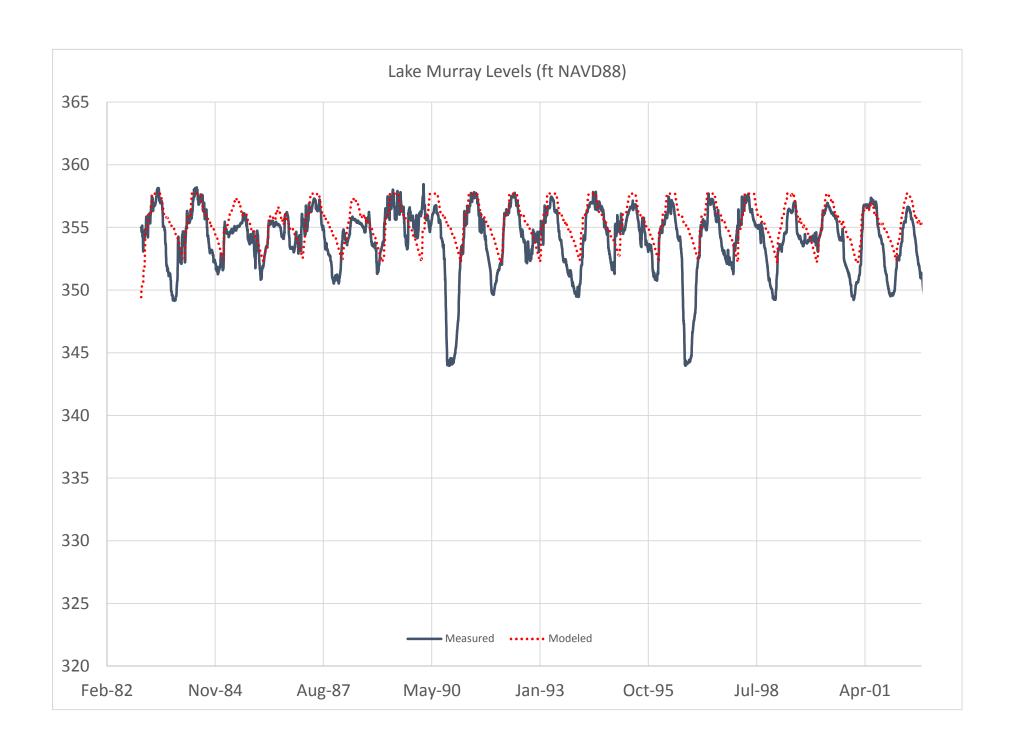




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Annual 7 day Low Flows: Modeled

	SLD04 Saluda	SLD 09 Saluda nr		SLD 18 Saluda at	SLD 19 Little Riv nr	SLD 21 Bush Riv at	SLD 22 Bush Riv nr	SLD 25 Saluda bl	SLD 27 Congaree at	SLD 29 Gills Crk at	SLD 32 Cedar Creek nr
		Ware Shoals			Silverstreet	Newberry	Prosperity	Lake Murray	Columbia	Columbia	Hopkins Flow
Year	Flow (CFS)	Flow (CFS)			Flow (CFS)	Flow (CFS)	Flow (CFS)	Flow (CFS)	Flow (CFS)	Flow (CFS)	(CFS)
1983	164	295	•	426	. ,				2667		•
1984	206	364		604							30
1985	161	287	109	431	20	14	24	335	2032	5	
1986	102	159	66	251	9	10	17	297	1240	4	12
1987	132	223	96	322	12	11	20	367	1833	12	20
1988	109	206	81	277	11	11	19	291	1959	4	
1989	189	325	119	450	21	14	23	256	1688	5	
1990	175	307	107	456	16	7	10			7	
1991	222	390	128	635	41	7	11	243	3052	31	
1992	268	442	116	483	26	6	10	314	2660	16	
1993	107	214	91	337	21	7	10	167	2344	10	26 26
1994	199	388	130	691	52		13	271	2964	7	26
1995	237	397	97	518	31	13	22	237	3403	16	35 26
1996	175	308	110	546	46		20			14	26
1997	143	261	122	481	35				2990		
1998	128	247	100	414	38		20		2738		29
1999	54	105		241	13		14		1912	. 8	
2000	68	130		255	7	_	10		1455	8	
2001	95	177	77	264	6		9		1683	6	
2002	56	96		231	4		10			4	7
2003	283	462	173	791	37		17			13	
2004	240	410		584	16		10			6	
2005	253	375		616	18		9			5	
2006	139	224		441	19		13			5	
2007	77	126		264	5		6			1	9
2008	48	84		210	4	-				7	
2009	116	184		293	11	6				4	11
2010	149	241		309	13		11			4	11
2011	88	153		267	7		8			3	
2012	168	292	103	360			8			8	
2013	281	492	178	786	24	6	11			13	12

Annual 7 day Low Flows: Measured

	SLD04 Saluda	SLD 09 Saluda nr	SLD 12&13 Reedy nr	SLD 18 Saluda at	SLD 19 Little Riv nr	SLD 21 Bush Riv at	SLD 22 Bush Riv nr	SLD 25 Saluda bl	SLD 27 Congaree at	SLD 29 Gills Crk at	SLD 32 Cedar Creek nr
	nr Greenville		Waterloo	Chappells	Silverstreet	Newberry	Prosperity	Lake Murray	Columbia	Columbia	Hopkins Flow
Year		Flow (CFS)	Flow (CFS)	Flow (CFS)	Flow (CFS)	Flow (CFS)	Flow (CFS)	Flow (CFS)	Flow (CFS)	Flow (CFS)	(CFS)
1983	110W (Cl 3)	360			11011 (613)	11000 (C13)	11000 (C13)	401	2211	• •	
1984		419						439			25
1985		295						289	2077	5	
1986		61						248	1560		
1987		159	77	295				270	1734	12	
1988		135	65	305				239	1721	5	
1989		353	67	475				168	2630	5	
1990		270	84	409				248	1870	7	
1991	298	470	150	605	39	2	24	330	3296	32	
1992	289	403	95	362	24	2	16	469	3356	17	
1993	181	260	104	356	19	0	13	398	2729	10	
1994	304	471	105	617	51	8	23	417	3009	7	
1995	297	468	121	442	30	3	18	399	4027	16	
1996	227	357	149	590	46	3	19	480	2621	14	
1997	168	256	139	525	35	3	15	436	2981	12	
1998	124					1	17	674	2501	10	
1999	72	95			13	0	9	188	1192	8	
2000	53	101	73				6	171	1357	8	
2001	92	155					5	323	1521	6	
2002	55	51	_				4	437	1274	4	
2003	311	468			36		14	549	3703	14	
2004	232	417		538	_		8	345	2337	6	
2005	235	351			18		13	504	1957	5	
2006	131	193			19		11	346	1764	5	
2007	87	119			3		5	267	904	1	
2008	54				1		4	235	810		
2009	93				7		6	242	1419		
2010	150				_		5	470	1290		
2011	85						3	297	916		
2012	181	208			6		3	279	1280		
2013	355	563	143	448	23		14	701	3546	13	

Approximate 7Q10 Comparison - Modeled vs. Gaged

	SLD04 Saluda nr Greenville Flow (CFS)		SLD 12&13 Reedy nr Waterloo Flow (CFS)	SLD 18 Saluda at Chappells Flow (CFS)	SLD 19 Little Riv nr Silverstreet Flow (CFS)	SLD 21 Bush Riv at Newberry Flow (CFS)	SLD 22 Bush Riv nr Prosperity Flow (CFS)	SLD 25 Saluda bl Lake Murray Flow (CFS)	SLD 27 Congaree at Columbia Flow (CFS)	SLD 29 Gills Crk at Columbia Flow (CFS)	SLD 32 Cedar Creek nr Hopkins Flow (CFS)
Modeled:	58	126	63	251	5	7	9	241	1592	3.7	25
Gaged:	58	94	37	251	2	0	4	235	1192	3.8	18
%Diff:	0%	35%	73%	0%	242%	*	121%	2%	34%	-1%	*

^{*} Relatively few years (<10) available to make comparison

Appendix C

Guidelines for Representing Multi-Basin Water Users in SWAM



Appendix C

Guidelines for Representing Multi-Basin Water Users in SWAM

There are many examples in South Carolina of water users that access source waters in multiple river basins and/or discharge return flows to multiple basins. Since SWAM models for each major river basin are being developed, it is important to represent the multi-basin users concisely and clearly in the models. The following provides a recommended set of consistent guidelines to follow as each river basin model is developed. In all cases, the constructs should be documented in the basin reports and described in the model itself using the Comment boxes.

- 1. If a water user's primary source of supply and discharge locations are located with the given river basin, then this user should be explicitly included as a Water User object in that basin model.
 - a. If secondary sources are from outside of the basin, then these should be included using the "transbasin import" option in SWAM.
 - b. If a portion of the return flows are discharged to a different basin, then this should be incorporated by using the multiple return flow location option, with the exported portion represented by a specified location far downstream of the end of the basin mainstem (e.g. mile "999").
- 2. If only a water user's secondary source of supply (i.e., not the largest portion of overall supply) is located outside the river basin being modeled, then this should be represented as a water user with an "Export" identifier in the name (e.g. "Greenville Export") in the river basin model where the source is located.
 - a. For this object, set the usage values based on only the amount sourced from inside the basin (i.e. only that portion of demand met by in-basin water).
 - b. Set the return flow location for this use to a location outside of the basin (e.g. mainstem mile "999").
 - c. For future demand projection simulations, the in-basin portion of overall demand will need to be disaggregated from the total demand projection, likely by assuming a uniform percent increase.
- 3. If a portion of a water user's return flow discharges to a different basin than the primary source basin, then this portion of return flow should be represented as a Discharge object (e.g. named "Greenville Import") in the appropriate basin model.
 - a. Reported discharge data can be used to easily quantify this discharge for historical calibration simulations.
 - b. For future demand projection simulations, this discharge can be easily quantified by analyzing the return flow output for the primary (source water basin). See 1b.

above. However, the user will need to manually make the changes to the prescribed Discharge object flows in the model.



